

**CHESTER BOROUGH LAND USE BOARD
AGENDA
MAY 14, 2020
7:00 PM**

Please select the below link to pre-register for the following meeting. It will require the user to provide your name and email address to setup a free account.

Instructions to Join the following meeting remotely by Zoom.

When: May 14, 2020 07:00 PM Eastern Time (US and Canada)

Topic: Land Use Board meeting

Please click the link below to join the webinar:

<https://us02web.zoom.us/j/87626979893>

Or iPhone one-tap :

US: +13126266799,,87626979893# or +19294362866,,87626979893#

Or Telephone:

Dial(for higher quality, dial a number based on your current location):

US: +1 312 626 6799 or +1 929 436 2866 or +1 301 715 8592 or +1 346 248 7799 or +1 669 900 6833 or +1 253 215 8782

Webinar ID: 876 2697 9893

International numbers available: <https://us02web.zoom.us/j/87626979893>

CHESTER BOROUGH LAND USE BOARD
AGENDA
APRIL 9, 2020
7:00 PM

1. CALL TO ORDER

2. OPEN PUBLIC MEETINGS ACT

Adequate notice of this meeting of the Chester Borough Land Use Board was given as required by the "Open Public Meetings Act" as follows: notice was sent to the Observer Tribune and the Daily Record, posted on the bulletin board in the Borough Municipal Building and posted on the Borough website and filed with the Borough Clerk. The Land Use Board will hold a regular meeting by web-based platform providing remote access as. This meeting will be open to the public remotely. Details for public participation will be posted on the website at chesterborough.org. Public can register at Zoom.us and to join :

<https://us02web.zoom.us/j/87626979893> Webinar ID: 876 2697 9893

In the event any member of the public cannot access the public file documents or exhibits, and/or cannot access the Zoom web-based virtual meeting room, they may contact the Board Secretary at kbrown@chesterborough.org or 908-879-3660 x 2123

3. SALUTE TO THE FLAG

4. ROLL CALL

Janet Hoven
Kerry Brown
Stan Stevinson
Anita Rhodes
Adam Sorchini
Stanley Quintana, Alternate #2

Steven Warner, Board Attorney
Steve Bolio, Board Engineer

5. MINUTES

A. April 9, 2020

6. BUSINESS

**A. Tack Veterinary Holdings, LLC
Amended Site Plan
114 US Highway 206 North Block 103, Lot 51**

**B. The Car Wash at Chester, LLC
Amended Preliminary and Final Major Site Plan and Variance Application
45 Maple Avenue Block 131, Lot 5**

C. Emergent Provisions Ordinance

Ordinance to allow for such prompt municipal action by way of resolutions of limited duration to provide for temporary relief from the requirements of certain existing ordinances such as outside dining.

7. RESOLUTIONS

No resolutions at this time

8. COMMUNICATION/DISCUSSION ITEMS

9. PUBLIC COMMENT

10. ADJOURNMENT

CERTIFICATIONS/APPROVALS

MUNICIPALITY
 THIS PLAN IS HEREBY APPROVED BY THE PLANNING BOARD OF THE BOROUGH OF CHESTER, MORRIS COUNTY

DATE _____ BOARD CHAIRMAN

DATE _____ BOARD SECRETARY

DATE _____ BOROUGH ENGINEER

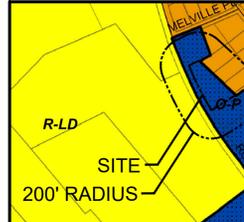
I HEREBY CERTIFY THAT I AM THE PRESENT OWNER OF THE REFERENCED PROPERTY AND THAT I CONSENT TO THE FILING OF THIS MINOR SITE PLAN WITH THE PLANNING BOARD OF THE BOROUGH OF CHESTER

DATE _____ DOUGLAS TACK

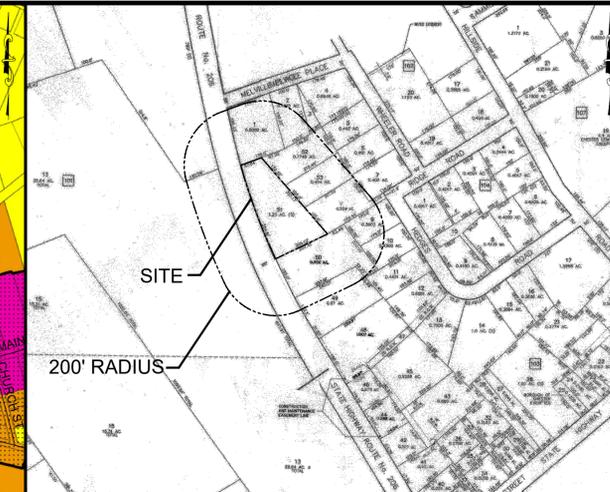
Legend

Zone

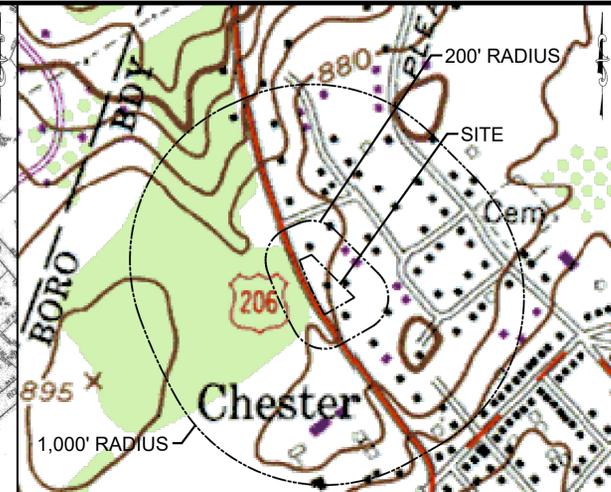
- R-LD: Residential/Low Density
- R-HD: Residential/High Density
- M-H: Mobile Home
- B-1: Historic Business
- B-2: Neighborhood Business
- B-3: Regional Commercial Business
- LBT: Limited Business Transitional
- O-P: Office Professional
- O-T: Office Transitional
- Industrial
- R-PLD: Residential-Planned Low Density Overlay
- Historic Preservation District Overlay
- Neighborhood Clustering/Lot Averaging Overlay



ZONING MAP (DISTRICT O-P & HISTORIC PRESERVATION)
 SCALE 1"=600'



TAX MAP (BLOCK 103, LOT 51, SHEETS 13, 14, 15)
 SCALE 1"=300'



USGS MAP (CHESTER QUAD)
 SCALE 1"=500'

ADDITIONAL PARTIES TO BE NOTIFIED

PROPERTY OWNER	ADDRESS
PUBLIC SERVICE ELECTRIC & GAS	80 PARK PLACE NEWARK, NJ 07101
NJ AMERICAN WATER CO.	PO BOX 5627 CHERRY HILL, NJ 08034

GENERAL NOTES:

- OWNER/APPLICANT:
TACK VETERINARY HOLDINGS, LLC
C/O DOUGLAS TACK
114 US HIGHWAY 206 NORTH
CHESTER, NJ 07930
PHONE: (908) 399 - 8076
- BOUNDARY AND TOPOGRAPHIC INFORMATION SHOWN HEREON WAS TAKEN FROM A PLAN ENTITLED "FINAL SURVEY/ SITE AS-BUILT FOR: LOT 51 BLOCK 103 BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY", PREPARED BY CIVIL ENGINEERING, INC., DATED 11/6/15 AND LAST REVISED 6/22/16.
- ADJACENT FEATURES FROM "BOUNDARY & TOPOGRAPHIC SURVEY", PREPARED BY CIVIL ENGINEERING, INC., ROBERT J. WESP, NJPLS No. 35891, DATED 2/26/10 OR LAST REVISED.
- THESE PLANS ARE NOT TO BE USED AS SURVEYS. REFER TO REFERENCE SOURCE FOR BOUNDARY AND TOPOGRAPHIC INFORMATION (NOTE # 2 ABOVE).
- ALL ELEVATIONS GIVEN ARE ON NAVD 88 DATUM.
- THE SUBJECT PARCEL, BLOCK 103 LOT 51, CONSISTS OF 51,974.3 S.F. (1.19 ACRES).

NOTES:

- SEE ENGINEERING DETAIL SHEETS FOR ALL SITE DETAILS AND SUPPORTING NOTES.
- THE LOCATION, TYPE, LINE, SIZE, DEPTH, ETC. OF ALL EXISTING UTILITIES, ARE APPROXIMATE. LOCATION OF SERVICE LATERALS MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION.
- THE CUMULATIVE INCREASE IN IMPERVIOUS COVERAGE OF THIS SITE PLAN AND PRIOR SITE PLAN (COMBINED) AS COMPARED TO PRE-EXISTING CONDITIONS IS LESS THAN 1/2 ACRE.

PRELIMINARY AND FINAL SITE PLANS BLACK RIVER VETERINARY HOSPITAL

114 US HIGHWAY 206 NORTH
 BLOCK 103, LOT 51, TAX MAP SHEET NO. 15
 CHESTER BOROUGH, MORRIS COUNTY, NEW JERSEY

PREPARED BY
ENGINEERING & LAND PLANNING ASSOCIATES, INC.
 140 WEST MAIN STREET, HIGH BRIDGE, NEW JERSEY 08829

NOTES:

- THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY APPROVAL. THIS SET OF PLANS SHALL NOT BE UTILIZED FOR CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH DRAWING HAS BEEN MARKED "ISSUED FOR CONSTRUCTION."

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CHESTER BOROUGH ZONING REQUIREMENTS			
ITEM	REQUIRED	EXISTING	PROPOSED
ZONING DISTRICT	O-P (OFFICE PROFESSIONAL ZONE)		
MIN. LOT AREA (AC.)	1 AC.	1.19 AC.	1.19 AC.
MIN. LOT WIDTH (FT)	150 FT	349.92 FT	349.92 FT
MIN. LOT DEPTH (FT)	150 FT	150.51 FT	150.51 FT
BUILDING SETBACKS FOR PRINCIPAL STRUCTURE			
FRONT YARD SETBACK (FT)	50 FT	> 50 FT	> 50 FT
SIDE YARD SETBACK - ONE SIDE (FT)	15 FT	18.4 FT	18.4 FT
SIDE YARD SETBACK - BOTH SIDES (FT)	30 FT	> 30 FT	> 30 FT
REAR YARD SETBACK (FT)	30 FT	37.4 FT	37.4 FT
MAX. BUILDING HEIGHT (FT) (STORY)	35 FT (2 1/2 STORIES)	< 35 FT (2 1/2 STORIES)	< 35 FT (2 1/2 STORIES)
BUILDING SETBACKS FOR ACCESSORY STRUCTURE			
MIN. FRONT YARD SETBACK (FT)	50 FT	N/A	N/A
MIN. SIDE YARD SETBACK (FT)	10 FT	N/A	N/A
MIN. REAR YARD SETBACK (FT)	10 FT	N/A	N/A
MAX. BUILDING LOT COVERAGE (%)	30 %	8.0 %	8.0 %
MAX. IMPERVIOUS COVERAGE (%)	60 %	33.1 %	34.8 %

PLAN INDEX	
SHEET No.	TITLE
1.	COVER SHEET
2.	LEGEND AND NOTES
3.	DEMOLITION PLAN
4.	SITE PLAN
5.	GRADING & UTILITY PLAN
6.	SESC PLAN
7.	LIGHTING & LANDSCAPING PLAN
8.	SESC NOTES
9.	SESC DETAILS
10.	CONSTRUCTION DETAILS

OUTSIDE AGENCY APPROVALS:

- NEW JERSEY HIGHLANDS COUNCIL (CONSISTENCY DETERMINATION)
- MORRIS COUNTY PLANNING BOARD

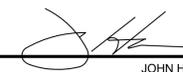
200 FT CERTIFIED OWNERS' LIST			
BLOCK	LOT	PROPERTY OWNER	ADDRESS
101	12	BOROUGH OF CHESTER	50 NORTH ROAD CHESTER, NJ 07930
101	13	TURKEY FARM ACQUISITION, LLC	3025 ROUTE 10 EAST MORRIS PLAINS, NJ 07950
103	1	HALL, ANDREW E	42 BARTLEY ROAD CHESTER, NJ 07930
103	2	BUCHANAN, EILEEN	20 MELVILLE PLACE, BOX 273 CHESTER, NJ 07930
103	6	LUONGO, JAMES / GILLILAND, HEATHER	3 WHEELER ROAD CHESTER, NJ 07930
103	7	SMITH, JEFFREY & SANDRA	1 WHEELER ROAD CHESTER, NJ 07930
103	9	SOMODY, GEORGE & SUE ANN	23 HEDGES ROAD CHESTER, NJ 07930
103	10	SIEGEL, JOSHUA	21 HEDGES ROAD CHESTER, NJ 07930
103	11	NINES, ROBERT & CLAIRE	19 HEDGES ROAD CHESTER, NJ 07930
103	49	STORMS, DONALD R JR	313 ROUTE 206 N, SUITE 2 CHESTER, NJ 07930
103	50	KRG HOLDINGS, LLC	104 ROUTE 206 CHESTER, NJ 07930
103	52	KALASHIAN, KRAIG J	29 HEDGES ROAD CHESTER, NJ 07930
103	53	FORGIONE, PATRICK / AMANDA	27 HEDGES ROAD CHESTER, NJ 07930
103	54	SCHMEAL, BRUCE & AMY	25 HEDGES ROAD CHESTER, NJ 07930

NOTICE TO BE SERVED FROM CERTIFIED LIST OBTAINED FROM THE BOROUGH.



140 WEST MAIN STREET HIGH BRIDGE, NJ 08829
 PH. 908-238-0544 FAX. 908-238-9572
 A PROFESSIONAL ASSOCIATION
 CERTIFICATE OF AUTHORIZATION NO.: 24GA28021500 EXP. 8/31/2020

NO.	REVISION	BY	DATE
1	REV. PER BOROUGH COMMENTS	EM	2/4/2020


 JOHN HANSEN
 PROFESSIONAL ENGINEER
 N.J. P.E. NO. 24GE04194500

PROJECT: **BLACK RIVER VETERINARY HOSPITAL SITE PLANS BLOCK 103, LOT 51 CHESTER BOROUGH MORRIS COUNTY NEW JERSEY**

TITLE: **COVER SHEET**

JOB NO.:	19003	DRAWING NO.:	1 10
SCALE:	N.T.S.		
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	01_COVER.DWG		
DATE:	05/01/2019		

STANDARD LEGEND

EXISTING	PROPOSED	EXISTING	PROPOSED
RIGHT-OF-WAY / PROPERTY		MONUMENT	
SETBACK LINE		ROW MONUMENT	
CURB		TEST PIT (NUMBER)	
DEPRESSED CURB		BORING (NUMBER)	
UNDERGROUND ELEC.		BUILDING TO BE DEMOLISHED	
UNDERGROUND GAS		BASE LINE	
UNDERGROUND WATER		HIGH POINT	
UNDERGROUND TELEPHONE		LOW POINT	
OVERHEAD WIRES		BENCH MARK	
UNDERGROUND SANITARY		WHITE ASH (FRAXINUS AMERICANA)	
FENCE		HORSE CHESTNUT (AESCULUS HIPPOCASTANUM)	
UNDERGROUND PIPE		EXISTING NORWAY MAPLE (ACER PLATANOIDES)	
TREE LINE		NORWAY SPRUCE (PILEA ABIES)	
CONTOUR		EXISTING TREE LOCATED OFF SITE (ALONG SOUTHERN PROPERTY LINE)	
BOLLARD		CONCRETE	
P.S.E & G. MANHOLE		PERVIOUS PAVEMENT	
SEWER MANHOLE		BITUMINOUS CONCRETE	
WATER MANHOLE			
MANHOLE			
BELL MANHOLE			
WATER METER			
ELEC. METER			
WATER VALVE			
GAS VALVE			
VALVE			
SAN. SEWER VENT			
CLEAN-OUT			
FIRE HYD.			
"A" SEWER INLET			
"B" SEWER INLET			
D.O.T. BOX			
TRAFF. LIGHT STANDARD			
TRAFF. SIGN			
UTILITY POLE			
OVERHEAD LIGHT			
ROOF DRAIN			
HEADWALL			
HEADWALL AND APRON			
DITCH			

STANDARD ABBREVIATIONS

AH., BK.	AHEAD, BACK	J.B.	JUNCTION BOX	RCP, R.C.P.	REINFORCED CONCRETE PIPE
\, B.L.	BASELINE	LT., RT.	LEFT, RIGHT	RMC, R.M.C.	RIGID NON-METALLIC CONDUIT
B.M.	BENCH MARK	L.O.P.	LIMIT OF PAVEMENT (PAVING)	RNM, R.N.M.C.	RIGHT OF WAY
B.T.	BELL TELEPHONE	L.O.M.	LIMIT OF MILLING	ROW, R.O.W.	RAILROAD
BIT., BITUM.	BITUMINOUS	M.B.	MAILBOX	R.R.	ROUTE
BLDG.	BUILDING	M.P.	MILE POST	RTE., RT.	SANITARY
. C.L.	CENTERLINE	MAX.	MAXIMUM	SAN.	SIDEWALK
C.I.P.	CAST IRON PIPE	MIN.	MINIMUM	SDWK.	STATE HIGHWAY DEPARTMENT
D.I.P.	DUCTILE IRON PIPE	NO.	NUMBER	S.H.D.	SHOULDER
CONC.	CONCRETE	N.T.S.	NOT TO SCALE	SHLD.	SURVEY LINE
CULV.	CULVERT	PAVT.	PAVEMENT	. S.L.	SUBBASE OUTLET DRAIN
D, DIA.	DIAMETER	PERF.	PERFORATED	STY.	TANGENT
D.C.	DROP CURB	P.G.L.	PROFILE GRADE LINE	T	TO BE ABANDONED
DE	DITCH EXCAVATION	J, P.L.	PROPERTY LINE, PROFILE LINE	TBA	TO BE REMOVED
DEP., DP	DEPRESSED CURB	PK	PARKER KAYLON MASONRY NAIL	TBR	TELEPHONE
DH	DRILL HOLE	POC, P.O.C.	POINT ON CURVE	TEL.	TEMPORARY
DWY	DRIVEWAY	POL, P.O.L.	POINT ON LINE	TEMP.	THICK
E.B., W.B.,	EASTBOUND, WESTBOUND	POT, P.O.T.	POINT ON TANGENT	THK., TH.	TYPICAL
N.B., S.B.	NORTHBOUND, SOUTHBOUND	PRC, P.R.C.	POINT OF REVERSE CURVE	TYP.	UNDERDRAIN
EL., ELEV.	ELEVATION	PROP.	PROPOSED	U.D.	UTILITY POLE
EXIST.	EXISTING	PT, P.T.	POINT OF TANGENCY	VAR.	VARIABLE, VARIES
GR.	GRATE	PVC, P.V.C.	POLYVINYL CHLORIDE PIPE,	WM	WATER METER
HT.	HEIGHT		POINT OF VERTICAL CURVATURE		
H.W.	HEADWALL		POINT OF VERTICAL INTERSECTION		
HYD.	HYDRANT		POINT OF VERTICAL TANGENCY, PAVEMENT		
INV.	INVERT		RADIUS		
IP	IRON PIN		REINFORCED CONCRETE CULVERT PIPE		

GENERAL NOTES:

- APPLICANT: TACK VETERINARY HOLDINGS, LLC
C/O DOUGLAS TACK
114 US HIGHWAY 206 NORTH
CHESTER, NJ 07930
PHONE: (908) 399 - 8076
- BOUNDARY AND TOPOGRAPHIC INFORMATION SHOWN HEREON WAS TAKEN FROM A PLAN ENTITLED "FINAL SURVEY/SITE AS-BUILT FOR: LOT 51 BLOCK 103 BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY", PREPARED BY CIVIL ENGINEERING, INC., DATED 11/6/15 AND LAST REVISED 6/22/16.
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- THESE PLANS ARE NOT TO BE USED AS SURVEYS. REFER TO REFERENCE SOURCES FOR BOUNDARY AND TOPOGRAPHIC INFORMATION (NOTE # 2 ABOVE).
- ALL CROSSWALKS, SIDEWALKS, AND CURB RAMPS WITHIN THE PROJECT LIMITS SHALL CONFORM TO ADA RULES AND REGULATIONS.
- THE CONTRACTOR SHALL CONFINE ALL CONSTRUCTION ACTIVITIES TO WITHIN THE LIMIT OF WORK AND/OR THE RIGHT-OF-WAY. ALL DISTURBED AREAS ARE TO BE RESTORED TO EXISTING CONDITIONS OR AS INDICATED IN THE CONTRACT DOCUMENTS.
- THE CONTRACTOR SHALL PROTECT ALL EXISTING STRUCTURES AND IMPROVEMENTS WITHIN THE PROJECT LIMITS AND RIGHT-OF-WAY. THESE FEATURES INCLUDE BUT ARE NOT LIMITED TO TREES, SHRUBS, LANDSCAPING, DRIVEWAYS, MAILBOXES, SIGNAGE, CURBING, SIDEWALKS, UTILITIES, JUNCTION BOXES, POLES, LIGHTING, HYDRANTS, VALVE BOXES, AND STRIPPING.
- THE CONTRACTOR SHALL OBTAIN ALL CONSTRUCTION PERMITS REQUIRED BY LOCAL, COUNTY OR STATE JURISDICTIONS PRIOR TO THE START OF CONSTRUCTION.
- THE CONTRACTOR SHALL NOTIFY THE BOROUGH AND MUNICIPAL ENGINEER'S OFFICE 72 HOURS PRIOR TO THE START OF WORK. THE CONTRACTOR SHALL NOTIFY ALL APPROPRIATE AGENCIES PRIOR TO THE START OF CONSTRUCTION AND AS NECESSARY THROUGH PROJECT COMPLETION.
- HOURS OF WORK SHALL BE RESTRICTED TO ORDINANCE REQUIREMENTS, UNLESS OTHERWISE PERMITTED BY TOWNSHIP.
- THE CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS BEFORE BEGINNING CONSTRUCTION.
- THE CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF THE PROJECT SITE, CONTRACTOR PROPERTY, EQUIPMENT, AND WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR CLEANING STREETS OF CONSTRUCTION DIRT AND DEBRIS AT CLOSE OF EACH WORK DAY.
- ANY DAMAGE TO THE PUBLIC ROAD DURING CONSTRUCTION SHALL BE REPLACED TO THE SATISFACTION OF THE STATE PRIOR TO ISSUANCE OF A CERTIFICATE OF OCCUPANCY.
- PRIOR TO CONSTRUCTION, THE CONTRACTOR, ALONG WITH CONCURRENCE FROM THE OWNER, SHALL DETERMINE HIS/HER LAY-DOWN AND/OR STAGING AREA LOCATIONS.
- TRAFFIC INGRESS AND EGRESS FOR DRIVEWAYS AND PEDESTRIAN ACCESS FACILITIES SHALL BE MAINTAINED THROUGHOUT CONSTRUCTION.
- PAVED SURFACES, PAVEMENT MARKERS AND MARKINGS SHALL BE PROTECTED FROM DAMAGE BY TRACKED EQUIPMENT.
- PROPERTY MARKERS DISTURBED DURING CONSTRUCTION ARE TO BE REPLACED BY A REGISTERED PUBLIC LAND SURVEYOR FOR THE ORIGINAL PROPERTY OWNER AT NO COST TO OWNER.
- CONSTRUCTION STAKING WILL BE PROVIDED BY THE CONTRACTOR UNLESS OTHERWISE AGREED TO WITH THE APPLICANT. TWO COPIES OF STAKING NOTES TO BE PROVIDED TO THE ENGINEER PRIOR TO CONSTRUCTION.
- THE CONTRACTOR SHALL MAINTAIN UPDATED RED-LINED RECORD DRAWINGS ON SITE FOR INSPECTION BY THE ENGINEER.
- MAINTENANCE AND CLEAN-UP OF THE PROJECT IS REQUIRED FOR THE PROJECT LIMITS AND DURATION, REGARDLESS OF THE CONTRACTOR'S SCOPE OF ACTIVITIES WITHIN THE PROJECT LIMITS.
- THE REMOVAL OF ANY ABANDONED UTILITIES REQUIRED TO COMPLETE THE WORK SHALL BE INCIDENTAL AND NO SEPARATE PAYMENT SHALL BE MADE.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO STOCKPILE NECESSARY MATERIAL ON-SITE OR AT A SECURED OFF-SITE LOCATION AT NO ADDITIONAL EXPENSE TO THE OWNER.
- THESE DRAWINGS DO NOT INCLUDE THE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY. THE CONTRACTOR SHALL BE RESPONSIBLE TO INSURE "ON THE JOB" SAFETY FOR HIS EMPLOYEES, EMPLOYEES OF THE OWNER AND ALL OTHER PERSONS HAVING AUTHORIZED OR UNAUTHORIZED ACCESS TO THE WORK AND THE PUBLIC. CONTRACTOR SHALL PERFORM HIS WORK IN A SAFE MANNER AND IN COMPLIANCE WITH ALL APPLICABLE LOCAL, STATE AND FEDERAL REGULATIONS.
- THE LOCATION, TYPE, SIZE, DEPTH, ETC. OF ALL EXISTING UTILITIES ARE APPROXIMATE AND MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR AND AT HIS OWN EXPENSE PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY ALL UTILITY COMPANIES OPERATING WITHIN THE CONSTRUCTION SITE 3 DAYS PRIOR TO COMMENCEMENT OF ANY EXCAVATION FOR ACCURATE FIELD LOCATIONS. FOR UTILITY MARKOUT, CALL 8-1-1 OR [FOR NJ, 1-800-272-1000]. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE DESIGN ENGINEER OF ALL DEVIATIONS OF SITE CONDITIONS AND/OR IF DEPARTURES FROM THE APPROVED DESIGN BECOME NECESSARY DUE TO SUCH DEVIATIONS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL LINES, ELEVATIONS, AND MEASUREMENTS, EXERCISING PRECAUTION TO VERIFY ALL DIMENSIONS SHOWN ON DRAWING.
- THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE DESIGN ENGINEER SHOULD DEPARTURES FROM THE APPROVED DESIGN BECOME NECESSARY DUE TO CIRCUMSTANCES WHICH ARISE DURING CONSTRUCTION.
- ITEMS NOT SPECIFIED BUT NECESSARY FOR PROPER CONSTRUCTION SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR THE SAME AS IF SPECIFIED.
- THE OWNER IS RESPONSIBLE FOR MAINTAINING BEST MANAGEMENT PRACTICES FOR STORAGE OF DE-ICING MATERIALS. DE-ICING MATERIALS SHALL BE STORED IN ACCORDANCE WITH ALL LOCAL, STATE, & FEDERAL REGULATIONS.

SITE/CIVIL NOTES:

- THE CONTRACTOR IS RESPONSIBLE FOR ENSURING PROPER DRAINAGE OF THE PROJECT SITE AND ALL UPGRADIENT AREAS TO THE SITE.
- CATCHBASIN GRATE AND HEADERS SHALL BE RESET AS REQUIRED TO MATCH FINISH GRADE ELEVATIONS, UNLESS OTHERWISE NOTED OR DIRECTED BY THE ENGINEER. PLACEMENT GRATES AND HEADERS SHALL BE ALIGNED WITH ADJACENT CURBING AND PAVEMENT.
- MILLINGS, STONE, SOIL, CONSTRUCTION DEBRIS, AND ALL OTHER RELATED MATERIALS SHALL BECOME THE PROPERTY OF THE CONTRACTOR FOR OFF-SITE DISPOSAL.
- INADEQUATE INSPECTION OF WORKMANSHIP SHALL NOT RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY TO PERFORM THE WORK IN ACCORDANCE WITH APPLICABLE PLANS, SPECIFICATIONS AND REGULATIONS AND TO OBTAIN WRITTEN APPROVAL OF MUNICIPAL OFFICIALS, AND ACCEPTANCE OF THE CONSTRUCTION BY THE OWNER.
- ALL CONCRETE USED FOR SITE WORK SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4,000 PSI. OR AS NOTED ELSE WHERE IN DRAWINGS.
- THE CONTRACTOR SHALL NOTIFY ALL UTILITY COMPANIES INDIVIDUALLY AND SHALL NOTIFY THE UNDERGROUND UTILITIES NOTIFICATION SERVICE AT LEAST 72 HOURS PRIOR TO COMMENCEMENT OF ANY EXCAVATION BY CALLING [1-800-272-1000 IN NJ].
- SUBBASE MATERIAL FOR SIDEWALKS, CURB, OR ASPHALT SHALL BE FREE OF ORGANIC AND OTHER UNSUITABLE MATERIALS. SHOULD SUBBASE BE DEEMED UNSUITABLE, SUBBASE IS TO BE REMOVED AND FILLED WITH APPROVED FILL MATERIAL COMPACTED TO 95% OPTIMUM DENSITY (AS DETERMINED BY MODIFIED PROCTOR METHOD).
- ALL CONTRACTORS WORKING ON THIS PROJECT SHALL COMPLY WITH THE REQUIREMENTS OF [29 CFR 1926 OSHA CONSTRUCTION INDUSTRY REGULATIONS & STANDARDS].
- ALL CONTRACTORS WORKING ON THIS PROJECT SHALL BE RESPONSIBLE FOR ENSURING THAT ALL CONSTRUCTION ACTIVITIES RELATED TO THIS PROJECT ARE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE OSHA (OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION) STANDARDS.
- NO EXCAVATION OR FILL SHALL BE MADE WITH A FACE STEEPER THAN THREE (3) HORIZONTAL TO ONE (1) VERTICAL (3:1) UNLESS A RETAINING WALL, CONSTRUCTED IN ACCORDANCE WITH APPROVED STANDARDS IS PROVIDED TO SUPPORT THE FACE OF SLOPE OF SAID EXCAVATION OR FILL.
- BURYING OF TREES, STUMPS, OR CONSTRUCTION MATERIAL IS PROHIBITED. TREES AND STUMPS MAY BE CHIPPED OR GROUND AND SPREAD ON THE SITE.
- SOIL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE IN PLACE AND FUNCTIONING PRIOR TO ANY OTHER DISTURBANCE ON THE SITE.
- EXISTING UTILITIES SHOWN ON THESE PLANS HAVE BEEN DEVELOPED FROM UTILITY COMPANY AS-BUILTS AND OR FIELD SURVEY AT THE SITE. COMPLETENESS AND/OR ACCURACY CANNOT BE GUARANTEED. ALL CONTRACTORS WORKING ON THIS PROJECT SHALL COMPLY WITH THE REQUIREMENTS OF [NEW JERSEY ONE CALL] REGARDING NOTIFICATION OF UNDERGROUND UTILITY USERS PRIOR TO EXCAVATION.

NOTES:

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PH. 908-238-0544 FAX. 908-238-9572
A PROFESSIONAL ASSOCIATION
CERTIFICATE OF AUTHORIZATION NO.: 24GA28021500 EXP. 8/31/2020

NO.	REVISION	BY	DATE
1	REV. PER BOROUGH COMMENTS	EM	2/4/2020

2/4/2020
DATE

JOHN HANSEN
 PROFESSIONAL ENGINEER
 N.J. P.E. NO. 24GE04194500

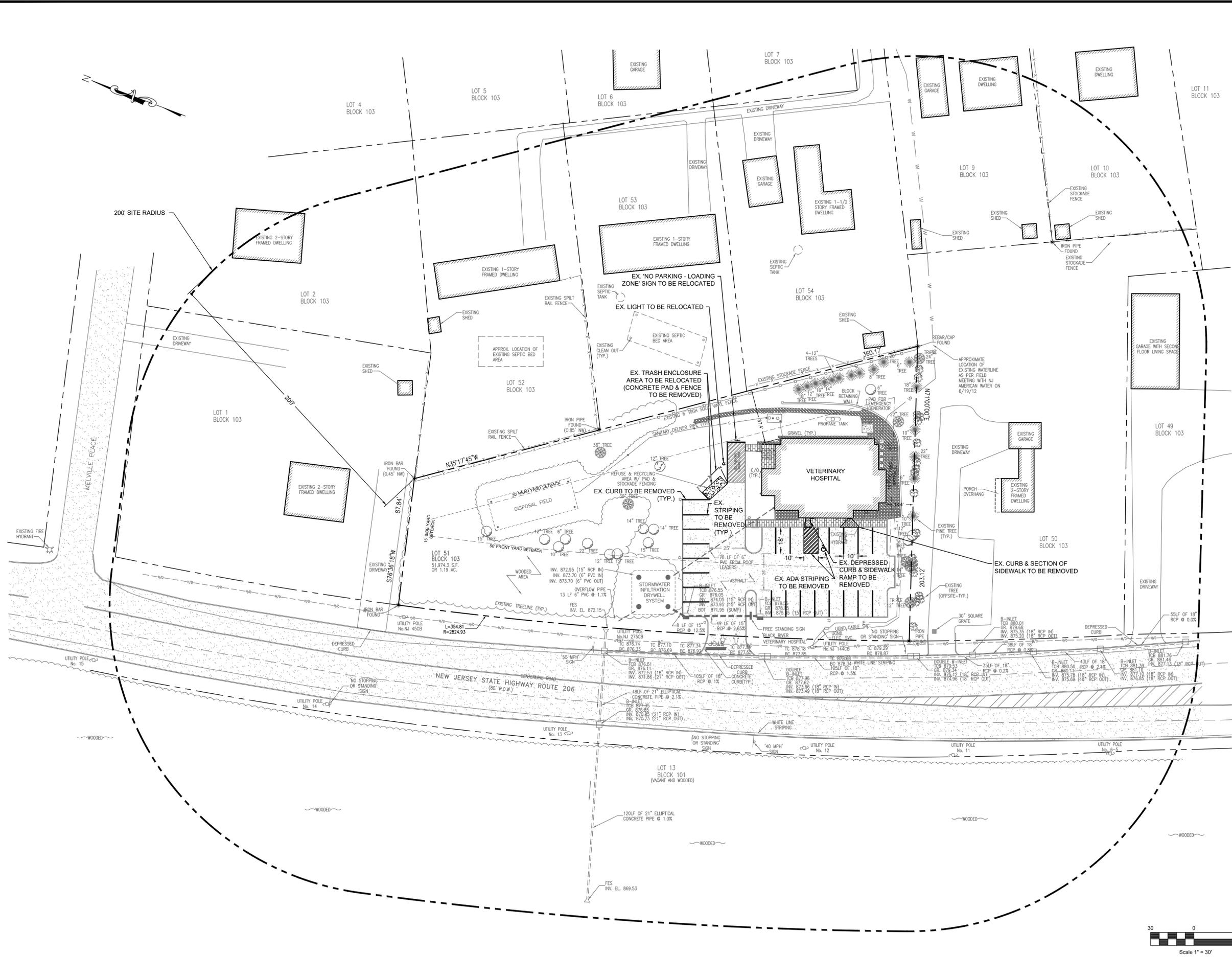
PROJECT:

**BLACK RIVER
VETERINARY HOSPITAL
SITE PLANS
BLOCK 103, LOT 51
CHESTER BOROUGH**

MORRIS COUNTY NEW JERSEY

LEGEND AND NOTES

JOB NO.:	19003	DRAWING NO.:	2 10
SCALE:	AS SHOWN		
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	02_LEGEND & NOTES.DWG		
DATE:	05/01/2019		



- NOTES:
- ADJACENT FEATURES FROM "BOUNDARY & TOPOGRAPHIC SURVEY", PREPARED BY CIVIL ENGINEERING, INC., ROBERT J. WESP, NJPLS No. 35891, DATED 2/26/10 OR LAST REVISED.
 - EXISTING ON-SITE FEATURES FROM "FINAL SURVEY/ SITE AS-BUILT FOR: LOT 51 BLOCK 103 BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY" PREPARED BY CIVIL ENGINEERING, INC., DATED 11/6/15 AND LAST REVISED 6/22/16.
 - ON-SITE FEATURES FROM PLAN REFERENCES REVISED PER SITE INSPECTION PERFORMED ON 1/25/19 & 4/10/19.
 - SEE SHEET 2 FOR ALL DEMOLITION AND SAFETY NOTES.

NOTES:

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1	REV. PER BOROUGH COMMENTS	EM	2/4/2020

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PROJECT:

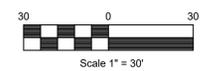
BLACK RIVER
 VETERINARY HOSPITAL
 SITE PLANS
 BLOCK 103, LOT 51
 CHESTER BOROUGH

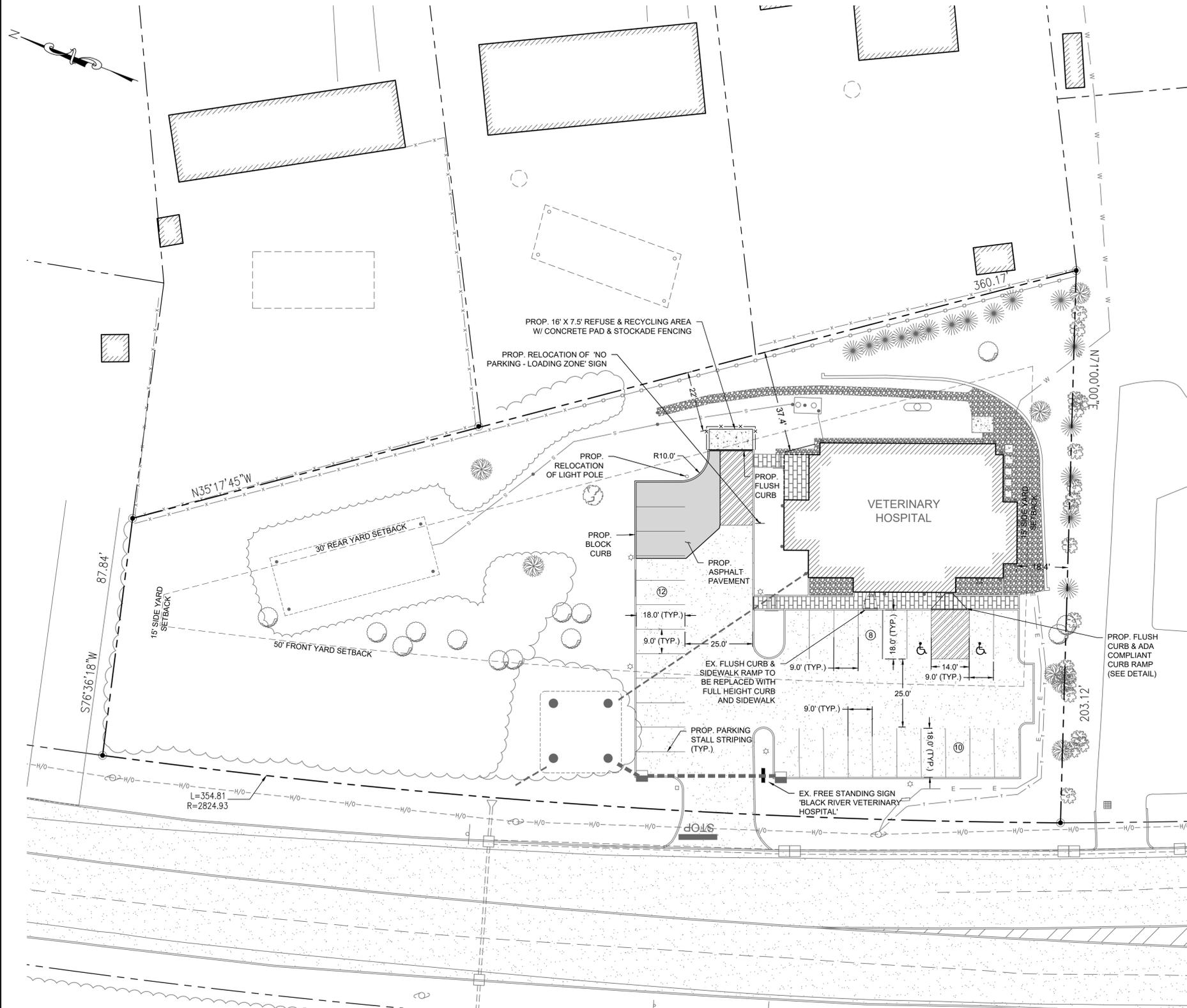
MORRIS COUNTY NEW JERSEY

TITLE:

DEMOLITION PLAN

JOB NO.:	19003	DRAWING NO.:	3
SCALE:	1"=30'		10
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	DEMO.DWG		
DATE:	05/01/2019		





CHESTER BOROUGH ZONING REQUIREMENTS			
ITEM	REQUIRED	EXISTING	PROPOSED
O-P (OFFICE PROFESSIONAL ZONE)			
ZONING DISTRICT			
MIN. LOT AREA (AC.)	1 AC.	1.19 AC.	1.19 AC.
MIN. LOT WIDTH (FT)	150 FT	349.92 FT	349.92 FT
MIN. LOT DEPTH (FT)	150 FT	150.51 FT	150.51 FT
BUILDING SETBACKS FOR PRINCIPAL STRUCTURE			
FRONT YARD SETBACK (FT)	50 FT	> 50 FT	> 50 FT
SIDE YARD SETBACK - ONE SIDE (FT)	15 FT	18.4 FT	18.4 FT
SIDE YARD SETBACK - BOTH SIDES (FT)	30 FT	> 30 FT	> 30 FT
REAR YARD SETBACK (FT)	30 FT	37.4 FT	37.4 FT
MAX. BUILDING HEIGHT (FT) (STORY)	35 FT (2 1/2 STORIES)	< 35 FT (2 1/2 STORIES)	< 35 FT (2 1/2 STORIES)
BUILDING SETBACKS FOR ACCESSORY STRUCTURE			
MIN. FRONT YARD SETBACK (FT)	50 FT	N/A	N/A
MIN. SIDE YARD SETBACK (FT)	10 FT	N/A	N/A
MIN. REAR YARD SETBACK (FT)	10 FT	N/A	N/A
MAX. BUILDING LOT COVERAGE (%)	30 %	8.0 %	8.0 %
MAX. IMPERVIOUS COVERAGE (%)	60 %	33.1 %	34.8 %

PREVIOUSLY APPROVED VARIANCES & WAIVERS PER RESOLUTION FOR APPLICATION NO. 2012-3, DECIDED AND MEMORIALIZED DECEMBER 13, 2012.

- VARIANCE(S):
- THE BOARD GRANTED APPROVAL OF THE VARIANCE FOR A FREESTANDING SIGN LOCATED 10 FEET FROM THE RIGHT-OF-WAY.
- DESIGN WAIVER(S):
- THE BOARD GRANTED THE RELIEF TO REDUCE THE RESIDENTIAL LANDSCAPE BUFFER TO 30.4 FEET.

NOTES:

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PREVIOUSLY APPROVED PARKING SCHEDULE

(PER RESOLUTION FOR APPLICATION NO. 2012-3, DECIDED AND MEMORIALIZED DECEMBER 13, 2012)
 (PER SHEET 4 OF 10 FROM PLAN SET ENTITLED "PRELIMINARY AND FINAL SITE PLANS FOR LOT 51 BLOCK 103 'BLACK RIVER VETERINARY HOSPITAL' SITUATED IN THE BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY", PREPARED BY CIVIL ENGINEERING, INC., DATED 7/6/12 AND LAST REVISED 12/30/12.)

ANIMAL HOSPITAL USE NOT CONTAINED IN ORDINANCE ARTICLE X, 163-80 PARKING AND LOADING REQUIREMENTS. PARKING CALCULATED AS FOLLOWS (CONSISTENT WITH PREVIOUS APPROVAL):

PARKING REQUIRED:
 PER ORDINANCE ARTICLE X, 163-80:
 5,243 S.F. OFFICE SPACE (1 PARKING STALL REQUIRED FOR EVERY 250 S.F. OFFICE SPACE) (NO CHANGE UNDER SUBJECT PROPOSAL) 21 TOTAL STALLS REQ.

SIMILAR PARKING REQUIREMENTS (FOR COMPARISON ONLY):
PARKING (FOR COMPARISON PURPOSES ONLY):

PER PREVIOUS SITE PLAN -
 4 EXAM ROOMS (ASSUME EACH OCCUPIED AND ONE CLIENT IN WAITING AREA FOR EACH) 8 STALLS
 +
 6 EMPLOYEES (4 EMPLOYEES + 2 DOCTORS ARE THE MAXIMUM AMOUNT AT THE SITE AT ANY ONE TIME, DURING BUSINESS OPERATING HOURS) (NO CHANGE UNDER SUBJECT PROPOSAL) 6 STALLS
 14 TOTAL STALLS REQ.

PER ORDINANCE ARTICLE X, 163-80 -
 5,243 S.F. MEDICAL SPACE (1 PARKING STALL REQUIRED FOR EVERY 180 S.F. MEDICAL SPACE) 29 TOTAL STALLS REQ.

ADA STALLS REQUIRED:
 26 - 50 STALLS TOTAL 2 ADA STALLS REQ.

PARKING PROVIDED:
 (28) 9' X 18' STALLS PROVIDED
 (2) 9' X 18' ADA W/ 14' VAN ACCESSIBLE ISLE PROVIDED
 30 TOTAL STALLS PROVIDED FOR SUBJECT APPLICATION

PREVIOUSLY APPROVED LOADING SCHEDULE

(PER RESOLUTION FOR APPLICATION NO. 2012-3, DECIDED AND MEMORIALIZED DECEMBER 13, 2012)
 (PER SHEET 4 OF 10 FROM PLAN SET ENTITLED "PRELIMINARY AND FINAL SITE PLANS FOR LOT 51 BLOCK 103 'BLACK RIVER VETERINARY HOSPITAL' SITUATED IN THE BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY", PREPARED BY CIVIL ENGINEERING, INC., DATED 7/6/12 AND LAST REVISED 12/30/12.)

LOADING AREA REQUIRED PER ORDINANCE ARTICLE X, 163-83A:
 250 S.F. LOADING AREA PER 25 LINEAR FEET OF PRINCIPAL BUILDING FRONTAGE

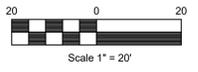
BUILDING FRONTAGE = 71.1'
 71.1' / 25' = 2.84 X 250 = 710 S.F. REQ. LOADING AREA

LOADING AREA PREVIOUSLY APPROVED:
 1 SPACE - 12' X 25' = 300 S.F.

* LOADING AREA COMBINED WITH ACCESS DRIVEWAY TO PARKING AREAS AND REFUSE AND RECYCLING AREA.

IMPERVIOUS MATERIALS

ITEM	EXISTING	PERCENTAGE	PROPOSED	PERCENTAGE
BUILDING	4,161 S.F.	8.0%	4,161 S.F.	8.0%
PARKING LOT & CURBING	10,470 S.F.		11,345 S.F.	
WALKWAYS, CONCRETE PADS, & GRAVEL	2,374 S.F.	25.1%	2,374 S.F.	26.8%
WALLS / SIGNS / MISC.	185 S.F.		185 S.F.	
TOTAL IMPERVIOUS COVERAGE	17,190 S.F.	33.1%	18,065 S.F.	34.8%



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NO.	REVISION	BY	DATE
1	REV. PER BOROUGH COMMENTS	EM	2/4/2020

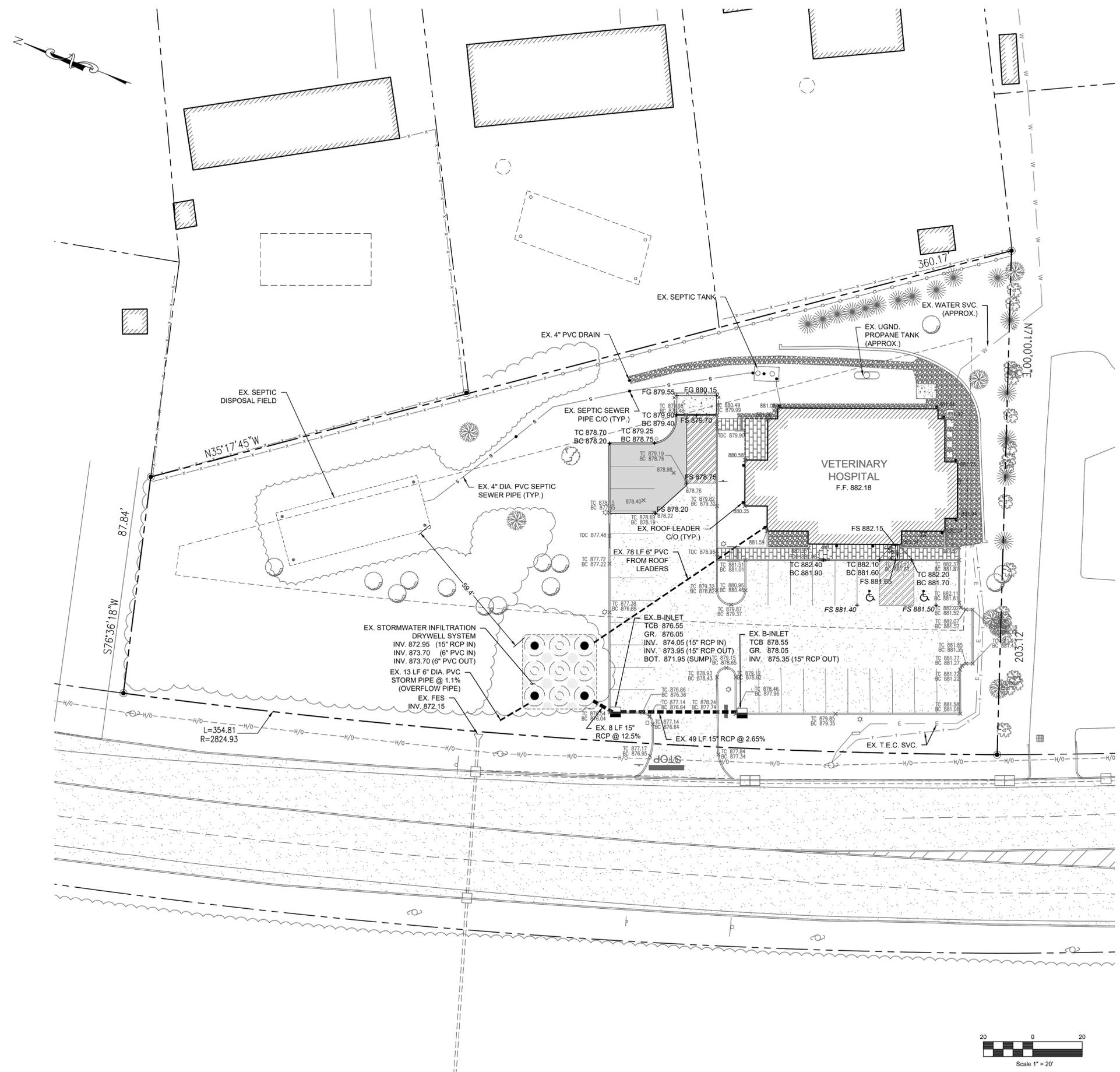
2/4/2020
DATE

JOHN HANSEN
PROFESSIONAL ENGINEER
N.J. P.E. NO. 24GE04194500

PROJECT:
BLACK RIVER VETERINARY HOSPITAL SITE PLANS BLOCK 103, LOT 51 CHESTER BOROUGH MORRIS COUNTY NEW JERSEY

TITLE:
SITE PLAN

JOB NO.:	19003	DRAWING NO.:	4 10
SCALE:	1"=20'		
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	SITE.DWG		
DATE:	05/01/2019		



NOTES:

1. ALL EXISTING SPOT ELEVATIONS SHOWN PER PLAN ENTITLED "FINAL SURVEY/SITE AS-BUILT FOR: LOT 51 BLOCK 103 BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY", PREPARED BY CIVIL ENGINEERING, INC., DATED 11/6/15 AND LAST REVISED 6/22/16.
2. ALL ELEVATIONS GIVEN ARE ON NAVD 88 DATUM.
3. ALL EXISTING ELEVATIONS AND SLOPES TO BE VERIFIED IN FIELD BY CONTRACTOR PRIOR TO CONSTRUCTION.

LEGEND

- 880.58
x EXISTING SPOT ELEVATION
- FS 878.32
+ EXISTING INTERPOLATED SPOT ELEVATION
(VERIFY IN FIELD)
- FS 878.32
+ PROPOSED SPOT ELEVATION
- 2.0%
— PROPOSED SLOPE

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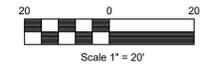
NO.	REVISION	BY	DATE
1	REV. PER BOROUGH COMMENTS	EM	2/4/2020

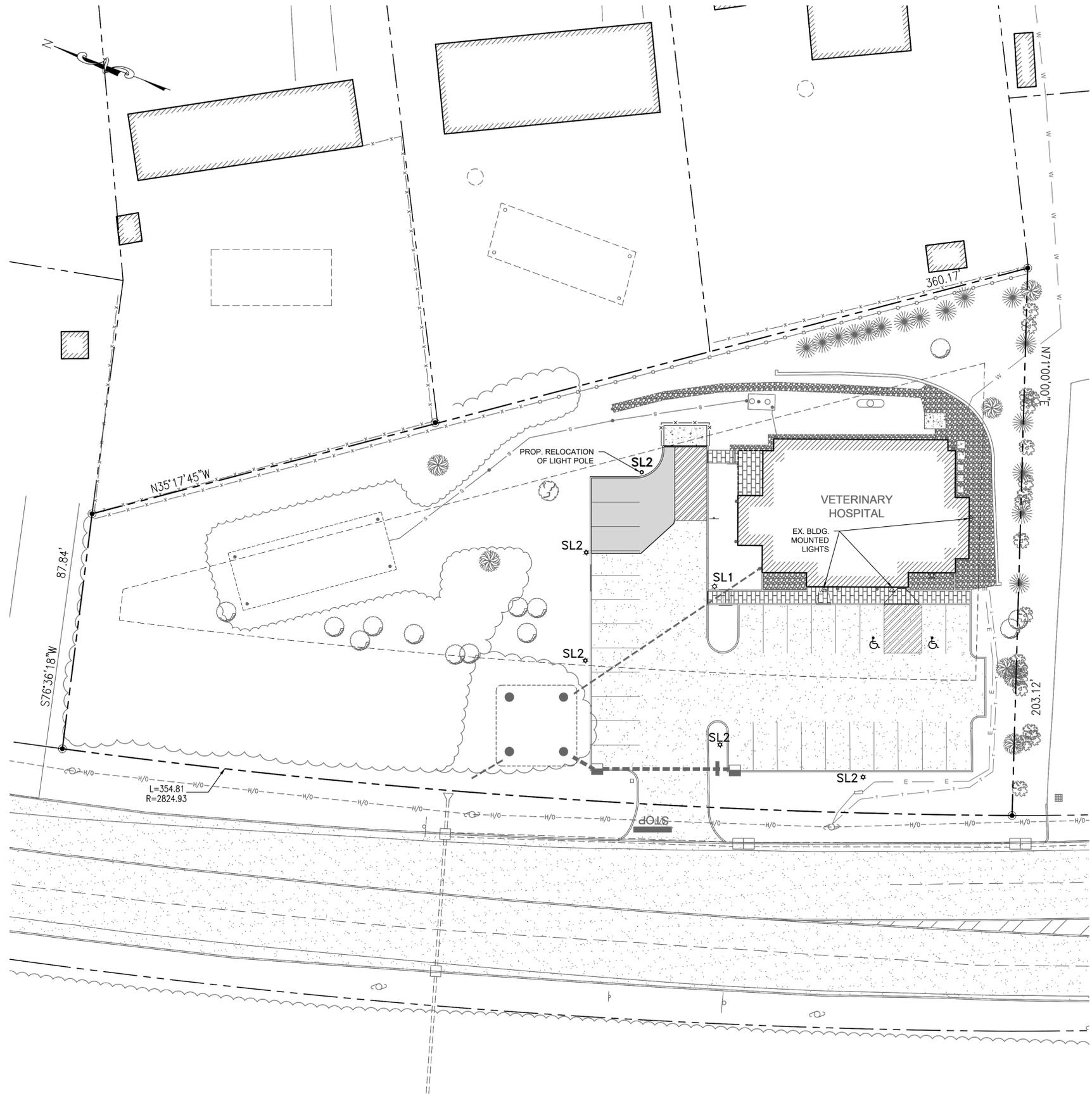
2/4/2020
 DATE  JOHN HANSEN
 PROFESSIONAL ENGINEER
 N.J. P.E. NO. 24GE04194500

PROJECT:
 BLACK RIVER
 VETERINARY HOSPITAL
 SITE PLANS
 BLOCK 103, LOT 51
 CHESTER BOROUGH
 MORRIS COUNTY NEW JERSEY

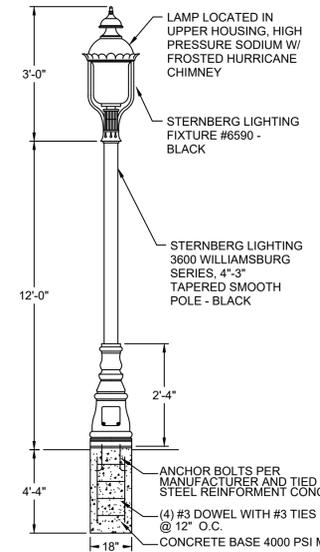
TITLE:
**GRADING & UTILITY
 PLAN**

JOB NO.:	19003	DRAWING NO.:	5 10
SCALE:	1"=20'		
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	GRAD_UTIL.DWG		
DATE:	05/01/2019		





LUMINAIRE SCHEDULE								
Symbol Label	Qty	Catalog #	Description	Lamp	File	Lumens	LLF	Watts
○ SL1	2	STERNBERG LIGHTING 6590/508/P17/36121 4/150HPS/R03/FHC/C11	LANERN CAGE ASSEMBLY w/CLEAR ACRYLIC LENSES VR = 2.2	150W HPS 6590R03H.IES		16000	0.80	150



- LIGHTING NOTES:**
- 1.) SITE LIGHTS ARE TO BE TURNED OFF AUTOMATICALLY AT 8PM PREVAILING TIME, EXCEPT FOR SECURITY LIGHTING AS DESIGNATED ON THE ARCHITECTURAL PLANS.
 - 2.) LIGHT FIXTURES ADJACENT TO CURB SHALL BE A MINIMUM OF THREE FEET FROM CURB FACE.
 - 3.) ALL POLE MOUNTED LIGHTING FIXTURES & LIGHTING CALCULATIONS PER RAE LYON OF LIBERTY LIGHTING GROUP - 100 PASSAIC AVE., CHATHAM, NJ 07928. PHONE (973) 701-0600. NO SUBSTITUTION ALLOWED FOR POLES & POLE MOUNTED FIXTURES.
 - 4.) ALL BUILDING & POLE MOUNTED LIGHTING TO BE DIRECTED DOWNWARD
 - 5.) POLES ON NORTHERLY SIDE OF PARKING AREA (SL1) TO BE EQUIPPED WITH HOUSE SIDE SHIELDS.

LIGHTING POWER DENSITY

MAXIMUM LIGHTING POWER DENSITY PER ORDINANCE = .11

LIGHTS ONLY

6 FIXTURES @ 150 WATTS/EA. = 900 WATTS TOTAL
 900 WATTS / 11,176 S.F. (PARKING LOT & DRIVEWAY AREA)
 = 0.08 LIGHTING POWER DENSITY

LIGHTS AND BALLASTS

6 FIXTURES @ 150 WATTS/EA. + 6 BALLASTS @ 35 WATTS/EA.
 = 1100 WATTS TOTAL
 1100 WATTS / 11,176 S.F. (PARKING LOT & DRIVEWAY AREA)
 = 0.09 LIGHTING POWER DENSITY

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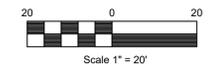
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PROJECT:
 BLACK RIVER
 VETERINARY HOSPITAL
 SITE PLANS
 BLOCK 103, LOT 51
 CHESTER BOROUGH
 MORRIS COUNTY NEW JERSEY

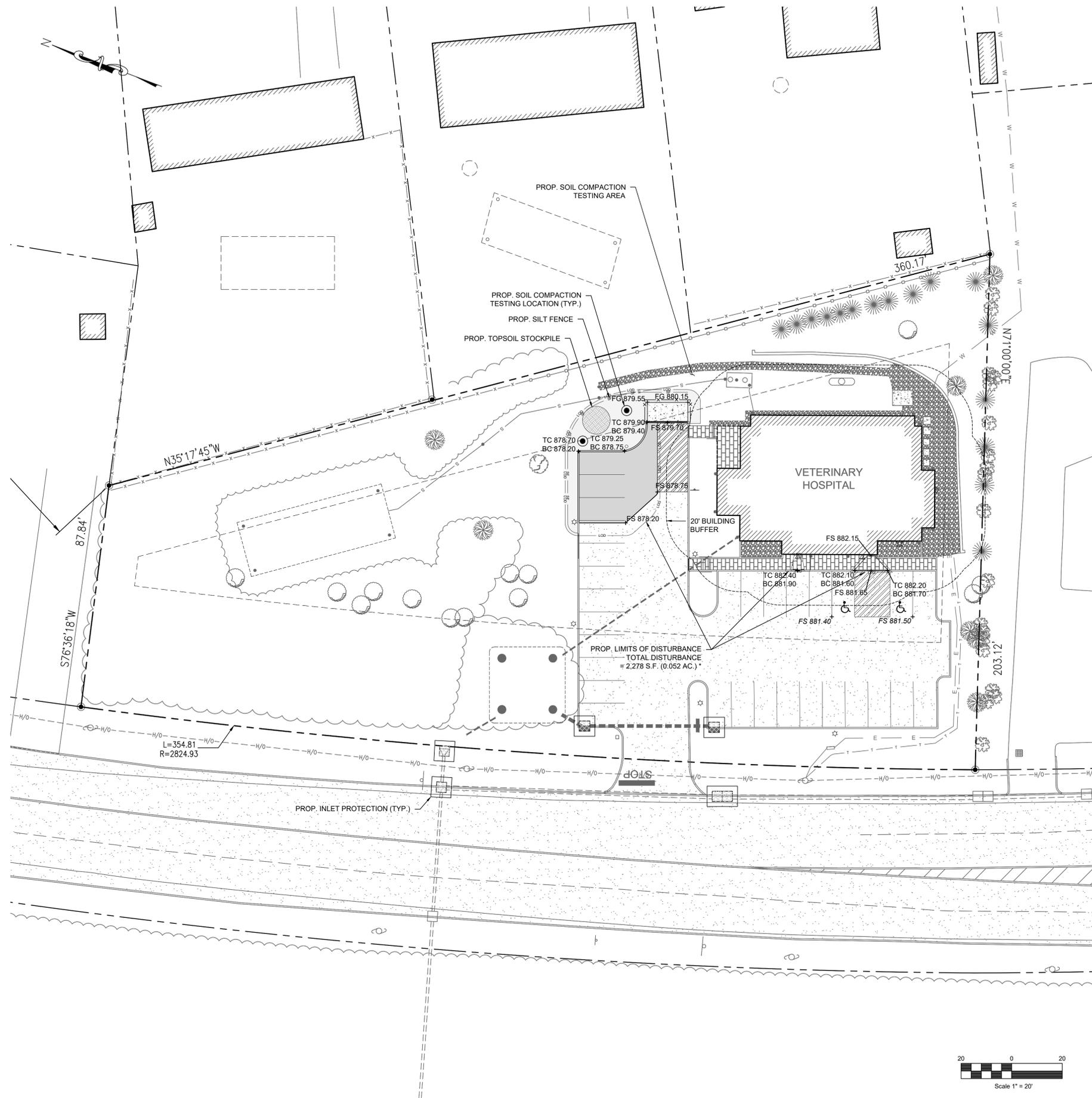
TITLE:
 LIGHTING &
 LANDSCAPING PLAN

JOB NO.:	19003	DRAWING NO.:	6
SCALE:	1"=20'		10
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	LANDSCAPE & LIGHTING.DWG		
DATE:	05/01/2019		

NOTE:
 EXISTING LIGHTING SHOWN PER PLAN ENTITLED "FINAL SURVEY/ SITE AS-BUILT FOR: LOT 51 BLOCK 103 BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY", PREPARED BY CIVIL ENGINEERING, INC., DATED 11/6/15 AND LAST REVISED 6/22/16.



- LANDSCAPING NOTES:**
1. THE EXISTING LANDSCAPING ON SITE IS TO REMAIN. A WAIVER IS REQUESTED FOR CHESTER BOROUGH ORDINANCE SECTION 163-47(A.33). A WAIVER HAS BEEN PREVIOUSLY GRANTED FOR ORDINANCE SECTION 163-49(G) PER RESOLUTION FOR APPLICATION NO. 2012-3, DECIDED AND MEMORIALIZED DECEMBER 13, 2012.
 2. DAMAGE TO EXISTING OR NEW WORK BY CONTRACTOR SHALL BE REPAIRED AT HIS EXPENSE.
 3. THE CONTRACTOR SHALL TAKE ALL PRECAUTIONS NECESSARY WITH REGARDS TO THE CARTING, STORING AND PLANTING OF MATERIALS TO PROTECT ADJACENT PAVEMENT AREAS.
 4. ALL TOPSOIL SHALL BE PER MSCD REQUIREMENTS. SEE SESC NOTES.
 5. TOPSOIL AND SEED ALL DISTURBED AREAS AS A RESULT OF ANY AND ALL CONSTRUCTION OR AREAS OF EQUIPMENT STORAGE EQUIPMENT. CONTRACTOR SHALL FIELD VERIFY AREAS OF SEEDING. PROVIDE TOPSOIL TO 6" DEPTH IN ALL LAWN AREA AND 20" DEPTH IN SHRUB BEDS AND GARDEN.
 6. ALL REMAINING DISTURBED AREAS SHALL BE SEEDED UNLESS NOTED OTHERWISE. SEE SEEDING PREPARATION NOTES. ALL LAWN AREAS TO BE FERTILIZED PRIOR TO FINAL SEEDING WITH A 10-6-4 FERTILIZER AT THE RATE OF 20 LBS/1000 SF. IN TWO APPLICATIONS.



LEGEND

- SF — SF — SF — SILT FENCE
- LOD — LOD — LOD — LIMIT OF DISTURBANCE
- TOPSOIL STOCKPILE
- INLET PROTECTION
- SOIL COMPACTION TESTING AREA
- SOIL COMPACTION TESTING LOCATION

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NOTES:

* THIS PROJECT IS EXEMPT FROM MORRIS COUNTY SOIL CONSERVATION DISTRICT APPROVAL SINCE THERE IS LESS THAN 5,000 S.F. OF DISTURBANCE PROPOSED.

ALL SOILS ON SITE ARE GLADSTONE GRAVELLY LOAM, 3-8% SLOPES (Gkaob) AS MAPPED PER WEB SOIL SURVEY.



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2/4/2020
DATE



JOHN HANSEN
PROFESSIONAL ENGINEER
N.J. P.E. NO. 24GE04194500

PROJECT:

**BLACK RIVER
VETERINARY HOSPITAL
SITE PLANS
BLOCK 103, LOT 51
CHESTER BOROUGH**

MORRIS COUNTY NEW JERSEY

TITLE:

**SOIL EROSION &
SEDIMENT CONTROL PLAN**

JOB NO.: 19003	DRAWING NO.:
SCALE: 1"=20'	7 10
DESIGNED: EM	
CHECKED: JH	
FILENAME: EROSION.DWG	
DATE: 05/01/2019	

SOIL EROSION AND SEDIMENT CONTROL NOTES:

- ALL SOIL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE INSTALLED IN ACCORDANCE WITH THE STANDARDS FOR SOIL EROSION AND SEDIMENT CONTROL IN NEW JERSEY, AND WILL BE IN PLACE PRIOR TO ANY MAJOR SOIL DISTURBANCE, OR IN THEIR PROPER SEQUENCE AND MAINTAINED UNTIL PERMANENT PROTECTION IS ESTABLISHED.
- ANY DISTURBED AREA THAT WILL BE LEFT EXPOSED FOR MORE THAN THIRTY (30) DAYS AND NOT SUBJECT TO CONSTRUCTION TRAFFIC SHALL IMMEDIATELY RECEIVE A TEMPORARY SEEDING. IF THE SEASON PROHIBITS TEMPORARY SEEDING, THE DISTURBED AREAS WILL BE MULCHED WITH STRAW OR HAY AND TACKED IN ACCORDANCE WITH THE NEW JERSEY STANDARDS. SEE NOTE 21 BELOW.
- PERMANENT VEGETATION IS TO BE ESTABLISHED ON EXPOSED AREAS WITHIN TEN (10) DAYS AFTER FINAL GRADING. MULCH IS TO BE USED FOR PROTECTION UNTIL VEGETATION IS ESTABLISHED. SEE NOTE 22 BELOW.
- IMMEDIATELY FOLLOWING INITIAL DISTURBANCE OR ROUGH GRADING, ALL CRITICAL AREAS (STEEP SLOPES, SANDY SOILS, WET CONDITIONS) SUBJECT TO EROSION WILL RECEIVE A TEMPORARY SEEDING IN ACCORDANCE WITH NOTE 21 BELOW.
- TEMPORARY DIVERSION BERMS ARE TO BE INSTALLED ON ALL CLEARED ROADWAYS AND EASEMENT AREAS. SEE THE DIVERSION DETAIL.
- PERMANENT SEEDING AND STABILIZATION TO BE IN ACCORDANCE WITH THE "STANDARDS FOR PERMANENT VEGETATIVE COVER FOR SOIL STABILIZATION COVER". SPECIFIED RATES AND LOCATIONS SHALL BE ON THE APPROVED SOIL EROSION AND SEDIMENT CONTROL PLAN.
- THE SITE SHALL AT ALL TIMES BE GRADED AND MAINTAINED SO THAT ALL STORMWATER RUNOFF IS DIVERTED TO SOIL EROSION AND SEDIMENT CONTROL FACILITIES.
- ALL SEDIMENTATION STRUCTURES (SILT FENCE, INLET FILTERS, AND SEDIMENT BASINS) WILL BE INSPECTED AND MAINTAINED DAILY.
- STOCKPILES SHALL NOT BE LOCATED WITHIN 50' OF A FLOODPLAIN, SLOPE, DRAINAGE FACILITY, OR ROADWAY. ALL STOCKPILE BASES SHALL HAVE A SILT FENCE PROPERLY ENTRENCHED AT THE TOE OF SLOPE.
- A STABILIZED CONSTRUCTION ACCESS WILL BE INSTALLED, WHENEVER AN EARTHEN ROAD INTERSECTS WITH A PAVED ROAD. SEE THE STABILIZED CONSTRUCTION ACCESS DETAIL AND CHART FOR DIMENSIONS.
- ALL NEW ROADWAYS WILL BE TREATED WITH SUITABLE SUBBASE UPON ESTABLISHMENT OF FINAL GRADE ELEVATIONS.
- PAVED ROADWAYS MUST BE KEPT CLEAN AT ALL TIMES.
- BEFORE DISCHARGE POINTS BECOME OPERATIONAL, ALL STORM DRAINAGE OUTLETS WILL BE STABILIZED AS REQUIRED.
- ALL DEWATERING OPERATIONS MUST BE DISCHARGED DIRECTLY INTO A SEDIMENT FILTER AREA. THE FILTER SHOULD BE COMPOSED OF A FABRIC OR APPROVED MATERIAL. SEE THE DEWATERING DETAIL.
- ALL SEDIMENT BASINS WILL BE CLEANED WHEN THE CAPACITY HAS BEEN REDUCED BY 50%. A CLEAN OUT ELEVATION WILL BE IDENTIFIED ON THE PLAN AND A MARKER INSTALLED ON THE SITE.
- DURING AND AFTER CONSTRUCTION, THE APPLICANT WILL BE RESPONSIBLE FOR THE MAINTENANCE AND UPKEEP OF THE DRAINAGE STRUCTURES, VEGETATION COVER, AND ANY OTHER MEASURES DEEMED APPROPRIATE BY THE DISTRICT. SAID RESPONSIBILITY WILL END WHEN COMPLETED WORK IS APPROVED BY THE MORRIS COUNTY SOIL CONSERVATION DISTRICT.
- ALL TREES OUTSIDE THE DISTURBANCE LIMIT INDICATED ON THE SUBJECT PLAN OR THOSE TREES WITHIN THE DISTURBANCE AREA WHICH ARE DESIGNATED TO REMAIN AFTER CONSTRUCTION ARE TO BE PROTECTED WITH TREE PROTECTION DEVICES. SEE THE TREE PROTECTION DETAIL.
- THE MORRIS COUNTY SOIL CONSERVATION DISTRICT MAY REQUEST ADDITIONAL MEASURES TO MINIMIZE ON SITE OR OFF SITE EROSION PROBLEMS DURING CONSTRUCTION.
- THE MORRIS COUNTY SOIL CONSERVATION DISTRICT MUST BE NOTIFIED, IN WRITING, AT LEAST 72 HOURS PRIOR TO ANY LAND DISTURBANCE, AND A PRE-CONSTRUCTION MEETING HELD.
- CONTRACTOR TO SET UP A MEETING WITH THE INSPECTOR FOR PERIODIC INSPECTIONS OF THE TEMPORARY SEDIMENT BASIN PRIOR TO AND DURING ITS CONSTRUCTION.
- TOPSOIL STOCKPILE PROTECTION
 - APPLY GROUND LIMESTONE AT A RATE OF 90 LBS PER 1000 SQ. FT.
 - APPLY FERTILIZER (10-20-10) AT A RATE OF 11 LBS. PER 1000 SQ. FT.
 - APPLY PERENNIAL RYEGRASS SEED AT 1 LB. PER 1000 SQ. FT. AND ANNUAL RYEGRASS AT 1 LB. PER 1000 SQ. FT.
 - MULCH STOCKPILE WITH STRAW OR HAY AT A RATE OF 90 LBS. PER 1000 SQ. FT.
 - APPLY A LIQUID MULCH BINDER OR TACK TO STRAW OR HAY MULCH.
 - PROPERTY ENTRENCH A SILT FENCE AT THE BOTTOM OF THE STOCKPILE.
- TEMPORARY STABILIZATION SPECIFICATIONS
 - APPLY GROUND LIMESTONE AT A RATE OF 90 LBS PER 1000 SQ. FT.
 - APPLY FERTILIZER (10-20-10) AT A RATE OF 11 LBS. PER 1000 SQ. FT.
 - APPLY PERENNIAL RYEGRASS SEED AT 1 LB. PER 1000 SQ. FT. AND ANNUAL RYEGRASS AT 1 LB. PER 1000 SQ. FT.
 - MULCH STOCKPILE WITH STRAW OR HAY AT A RATE OF 90 LBS. PER 1000 SQ. FT.
 - APPLY A LIQUID MULCH BINDER OR TACK TO STRAW OR HAY MULCH.
- PERMANENT STABILIZATION SPECIFICATIONS
 - APPLY TOPSOIL TO A DEPTH OF 5 INCHES (UNSETTLED)
 - APPLY GROUND LIMESTONE AT A RATE OF 90 LBS PER 1000 SQ. FT.
 - APPLY FERTILIZER (10-20-10) AT A RATE OF 11 LBS. PER 1000 SQ. FT.
 - APPLY HARD FESCUE SEED AT 2.7 LBS. PER 1000 SQ. FT. AND CREEPING RED FESCUE SEED AT 0.7 LBS. PER 1000 SQ. FT. AND PERENNIAL RYEGRASS SEED AT 0.25 LBS. PER 1000 SQ. FT.
 - MULCH STOCKPILE WITH STRAW OR HAY AT A RATE OF 90 LBS. PER 1000 SQ. FT.
 - APPLY A LIQUID MULCH BINDER OR TACK TO STRAW OR HAY MULCH.

* NOTE: 72 HOURS PRIOR TO ANY SOIL DISTURBANCE, NOTICE IN WRITING, SHALL BE GIVEN TO THE MORRIS COUNTY SOIL CONSERVATION DISTRICT AND A PRE-CONSTRUCTION MEETING HELD.

AGRONOMIC SPECIFICATIONS FOR LAWNS AND CONSTRUCTION SITES

- GENERAL**
- ALL DISTURBED AREAS THAT ARE NOT BEING GRADED, NOT UNDER ACTIVE CONSTRUCTION, OR NOT SCHEDULED TO BE PERMANENTLY SEEDED WITHIN 30 DAYS MUST BE TEMPORARILY STABILIZED AS PER SPECIFICATIONS BELOW.
 - ALL EXPOSED AREAS WHICH ARE TO BE PERMANENTLY VEGETATED, ARE TO BE SEEDED AND MULCHED WITHIN 10 DAYS OF FINAL GRADING.
 - MULCH ANCHORING IS REQUIRED AFTER MULCHING TO MINIMIZE LOSS BY WIND OR WATER. THIS IS TO BE DONE USING ONE OF THE METHODS (CRIMPING, LIQUID MULCH BINDERS, NETTINGS, ETC.) IN THE "STANDARDS FOR SOIL EROSION AND SEDIMENT CONTROL IN NEW JERSEY".
 - EXISTING WEEDY AND POORLY-VEGETATED AREAS WITH LESS THAN 80 PERCENT PERENNIAL GRASS COVER MUST RECEIVE PERMANENT STABILIZATION (AS SPECIFIED ON BACK).
 - ALL BAGS NEED TO BE SAVED FOR LIME, FERTILIZER, SEED, AND LIQUID MULCH BINDER (IF MULCH ANCHORING METHOD). SUCH PROOFS NEED TO BE SUBMITTED TO THE DISTRICT INSPECTOR FOR VERIFICATION OF MATERIALS AND QUANTITIES USED FOR ALL SEEDINGS.
 - AN ADDITIONAL FEE OF \$175.00 PER INSPECTION WILL BE ASSESSED TO THOSE SITES WHERE ADDITIONAL INSPECTIONS ARE NECESSITATED AS A RESULT OF NON-COMPLIANCE WITH THE APPROVED PLAN. THIS INCLUDES ADDITIONAL INSPECTIONS PERFORMED AFTER THE FAILURE OF AN INITIAL REPORT OF COMPLIANCE INSPECTION. THE ENTIRE SITE IS INSPECTED AT THE TIME OF A REQUEST FOR REPORT OF COMPLIANCE.

SEED-BED PREPARATION FOR ALL SEEDINGS

- SUB-SOIL PREPARATION:** IMMEDIATELY PRIOR TO SEEDING AND TOPSOIL APPLICATION, THE SURFACE SHOULD BE SCARIFIED TO A DEPTH OF 6" TO 12" WHERE THERE HAS BEEN SOIL COMPACTION (e.g. AREAS OF HEAVY CONSTRUCTION TRAFFIC). THIS PRACTICE IS TO BE APPLIED TO ALL COMPACTED AREAS WHERE THERE IS NO DANGER TO UNDERGROUND UTILITIES (CABLES, IRRIGATION SYSTEMS, ETC.).
- TOPSOILING:** AREAS TO BE SEEDED SHOULD HAVE A MINIMUM OF 5" OF FRIABLE, LOAMY, TOPSOIL FREE OF OBJECTIONABLE WEEDS, STONES, AND DEBRIS.
- FINAL GRADING:** GRADING IS TO BE SMOOTH OF RUTS AND FREE OF OBJECTIONABLE STONES, DEPRESSIONS, VEHICLE TRACKS, AND ROUGH EDGES. THERE IS TO BE POSITIVE DRAINAGE AWAY FROM ALL BUILDINGS AND DWELLINGS. REFUSE FROM SEEDBED PREPARATION (ROOTS, STICKS, STONES, CONSTRUCTION DEBRIS) MUST BE DISPOSED OF PROPERLY.

- LIMING/FERTILIZING:** APPLY LIMESTONE AND FERTILIZER TO SOIL TEST RECOMMENDATIONS:
- LIME IS TO BE APPLIED AT THE RECOMMENDED RATE. LIME MAY BE ANY PRODUCT AS TONS AS THE CCE CALCIUM CARBONATE EQUIVALENCY. PELLETIZED AND LIQUID PRODUCTS MAY BE PREFERRED BECAUSE OF THEIR LACK OF DUST AND EASE OF HANDLING BUT MUST MEET THE FORE-MENTIONED CRITERIA.
 - STARTER FERTILIZER, SPECIFIED AS 10-20-10, IS TO BE APPLIED AT 500 LBS. PER ACRE.
 - LIME AND FERTILIZER ARE TO BE WORKED INTO THE SOIL TO A DEPTH OF 4 INCHES.

TEMPORARY STABILIZATION WITH MULCH ONLY

STRAW MULCH (HAY MULCH MAY BE SUBSTITUTED IF APPROVED BY THE DISTRICT) IS TO BE SPREAD UNIFORMLY AT THE RATE OF 2 TO 2 1/2 TONS PER ACRE (TOTAL GROUND SURFACE COVERAGE). THIS PRACTICE IS LIMITED TO PERIODS WHEN VEGETATIVE COVER CANNOT BE ESTABLISHED DUE TO THE SEASON OR OTHER CONDITIONS. MULCH MUST BE ANCHORED IN ACCORDANCE WITH NEW JERSEY STANDARDS FOR SOIL EROSION AND SEDIMENT CONTROL. MULCH ALONE CAN ONLY BE USED FOR SHORT PERIOD AND WILL REQUIRE MAINTENANCE AND RENEWAL. OTHER MULCH MATERIALS MAY BE UTILIZED IF APPROVED BY THE DISTRICT.

TEMPORARY SEEDING

TEMPORARY SEEDING IS TO BE USED ON ALL DISTURBED AREAS WHERE PERMANENT STABILIZATION WILL NOT BE ACCOMPLISHED FOR A PERIOD OF UP TO 6 MONTHS.

PRODUCT	RATE	RECOMMENDED OPTIMUM SEEDING DATES
PERENNIAL RYEGRASS	100 LBS./ACRE	3/15-5/15 & 8/15-10/1
SPRING OATS	86 LBS./ACRE	3/15-6/1 & 8/1-10/1
WINTER CEREAL RYE	112 LBS./ACRE	8/1-11/15
WINTER BARLEY	96 LBS./ACRE	8/15-10/1
PEARL MILLET	20 LBS./ACRE	5/15-8/15
GERMAN OR HUNGARIAN MILLET	30 LBS./ACRE	5/15-8/15

STABILIZATION WITH SOD

STABILIZATION WITH SOD IS PERMITTED IN AREAS WHERE MAINTENANCE AND IRRIGATION ARE ADEQUATE TO INSURE PROPER ESTABLISHMENT AND LONGEVITY. SEEDBED PREPARATION IS TO BE CONSISTENT WITH ANY OTHER STABILIZATION REQUIREMENTS. (LIME AND FERTILIZER BAGS ARE TO BE RETAINED FOR DISTRICT INSPECTION.) ON SLOPES GREATER THAN 3 TO 1, SOD MUST BE PROPERLY ANCHORED TO THE SLOPE IN ACCORDANCE WITH THE STANDARDS FOR SOIL EROSION AND SEDIMENT CONTROL.

PERMANENT SEEDING

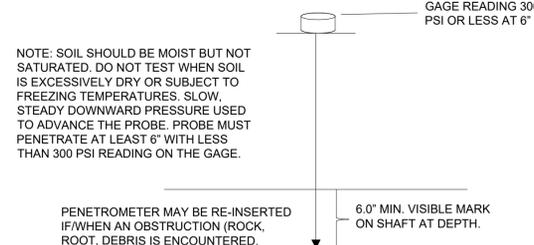
- SEED IS TO BE INCORPORATED INTO THE SOIL TO A DEPTH OF 1/4"-1/2".
- LAWN SEEDINGS ARE TO BE A MIXTURE OF BLUEGRASS, TURF-TYPE FESCUES, AND TURF-TYPE PERENNIAL RYEGRASSES TO INSURE LONGEVITY, TOLERANCE, AND DURABILITY. NO SEED SHALL BE ACCEPTED WITH A GERMINATION TEST DATE OF MORE THAN 12 MONTHS OLD UNLESS RETESTED.
- PROFESSIONAL SEED MIXTURES ARE RECOMMENDED RATHER THAN MIXING SEEDS YOURSELF.
- SEED MIXTURE (AS SPECIFIED BELOW) IS TO BE APPLIED AT A MINIMUM RATE OF 200 LBS. PER ACRE OF PERENNIAL SEED.
- OPTIMUM SEEDING PERIOD FOR MORRIS COUNTY IS FROM MARCH 1 TO MAY 15 AND AUGUST 15 TO OCTOBER 1. OUTSIDE OF THOSE PERIODS, THE SEEDING RATES ARE TO BE INCREASED BY 50 (i.e.: 300 LBS. PER ACRE OF PERENNIAL SEED INSTEAD OF THE REQUIRED 200 LBS. PER ACRE DURING OPTIMUM PERIODS).
- SEEDINGS SHOULD RECEIVE AN APPLICATION OF FERTILIZER SUCH AS 10-10-10 OR EQUIVALENT AT 400 LBS. PER ACRE APPROXIMATELY 6 MONTHS AFTER FIRST APPLICATION.

SEEDING MIXTURE FOR GENERAL SEEDING

FROM THE "STANDARDS FOR SOIL EROSION AND SEDIMENT CONTROL IN NEW JERSEY" (TABLE 4-3):

MIX #12	TURF-TYPE TALL FESCUE (BLEND OF 3 CULTIVARS)
	SEEDING RATE 350 LBS./ACRE
	OPTIMUM SEEDING DATES: AUGUST 15 TO OCTOBER 15

HANDHELD SOIL PENETROMETER TEST



SIMPLIFIED TESTING METHODS

PENETROMETER MAY BE RE-INSERTED IF/WHEN AN OBSTRUCTION (ROCK, ROOT, DEBRIS IS ENCOUNTERED).

PROBING WIRE TEST- 15.5 GA STEEL WIRE (SURVEY FLAG)

DUST CONTROL NOTES

THE FOLLOWING METHODS SHOULD BE CONSIDERED FOR CONTROLLING DUST:

- MULCHES
- VEGETATIVE COVER
- SPRAY-ON ADHESIVES - ON MINERAL SOILS (NOT EFFECTIVE ON MUCK SOILS). KEEP TRAFFIC OFF THESE AREAS.

DUST CONTROL MATERIALS

MATERIAL	WATER DILUTION	TYPE OF NOZZLE	APPLY GAL/AC
ANIONIC ASPHALT EMULSION	7:1	COARSE SPRAY	1200
LATEX EMULSION	12.5:1	FINE SPRAY	235
RESIN IN WATER	4:1	FINE SPRAY	300
POLYACRYLAMIDE (PAM) - SPRAY ON POLYACRYLAMIDE (PAM) - DRY SPRAY		APPLY ACCORDING TO MANUFACTURERS'S INSTRUCTIONS MAY ALSO BE USED AS AN ADDITIVE TO SEDIMENT BASINS TP FLOCCULATE AND PRECIPITATE SUSPENDED COLLOIDS.	
ACIDULATED SOY BEAN SOAP STICK	NONE	COARSE SPRAY	1200

TILLAGE - TO ROUGHEN SURFACE AND BRING CLODS TO THE SURFACE. THIS IS A TEMPORARY EMERGENCY MEASURE WHICH SHOULD BE USED BEFORE SOIL BLOWING STARTS. BEGIN PLOWING ON WINDWARD SIDE OF SITE. CHISEL-TYPE PLOWS SPACED ABOUT 18 INCHES APART, AND SPRING-TOOTHED HARROWS ARE EXAMPLES OF EQUIPMENT WHICH MAY PRODUCE THE DESIRED EFFECT.

SPRINKLING - SITE IS SPRINKLED UNTIL THE SURFACE IS WET.

BARRIERS - SOLID BOARD FENCES, SNOW FENCES, BURLAP FENCES, CRATE WALLS, BALES OF HAY, AND SIMILAR MATERIAL CAN BE USED TO CONTROL AIR CURRENTS AND SOIL BLOWING.

CALCIUM CHLORIDE - SHALL BE IN THE FORM OF LOOSE, DRY GRANULATES OF FLAKES FINE ENOUGH TO FEED THROUGH COMMONLY USED SPREADERS AT A RATE THAT WILL KEEP SURFACE MOIST BUT NOT CAUSE POLLUTION OR PLANT DAMAGE. IF USED ON STEEPER SLOPES, THEN USE OTHER PRACTICES TO PREVENT WASHING INTO STREAMS, OR ACCUMULATION AROUND PLANTS.

STONE - COVER SURFACE WITH CRUSHED STONE OR COARSE GRAVEL.

MULCHING:

MULCHING IS REQUIRED ON ALL SEEDING. MULCH WILL PROTECT AGAINST EROSION BEFORE GRASS IS ESTABLISHED AND WILL PROMOTE FASTER AND EARLIER ESTABLISHMENT. THE EXISTENCE OF VEGETATION SUFFICIENT TO CONTROL SOIL EROSION SHALL BE DEEMED COMPLIANT WITH THIS MULCHING REQUIREMENT.

- STRAW OR HAY. UNROTTED SMALL GRAIN STRAW, HAY FREE OF SEEDS, TO BE APPLIED AT THE RATE OF 1-1/2 TO 2 TONS PER ACRE (70 TO 90 POUNDS PER 1,000 SQUARE FEET), EXCEPT THAT WHERE A CRIMPER IS USED INSTEAD OF A LIQUID MULCH-BINDER (TACKIFYING OR ADHESIVE AGENT), THE RATE OF APPLICATION IS 3 TONS PER ACRE. MULCH CHOPPER-BLOWERS IS NOT GRIND THE MULCH. HAY MULCH IS NOT RECOMMENDED FOR REESTABLISHING FINE TURF OR LAWNS DUE TO THE PRESENCE OF WEED SEED.

APPLICATION: SPREAD MULCH UNIFORMLY BY HAND OR MECHANICALLY SO THAT AT LEAST 85% OF THE SOIL SURFACE WILL BE COVERED. FOR UNIFORM DISTRIBUTION OF HAND-SPREAD MULCH, DIVIDE AREA INTO APPROXIMATELY 1,000 SQUARE FEET SECTIONS AND DISTRIBUTE 70 TO 90 POUNDS WITHIN EACH SECTION. ANCHORING SHALL BE ACCOMPLISHED IMMEDIATELY AFTER PLACEMENT TO MINIMIZE LOSS BY WIND OR WATER. THIS MAY BE DONE BY ONE OF THE FOLLOWING METHODS, DEPENDING UPON THE SIZE OF THE AREA, STEEPNESS OF SLOPE, AND COSTS.

- PEG AND TWINE. DRIVE 8 TO 10 INCH WOODEN PEGS TO WITHIN 2 TO 3 INCHES OF THE SOIL SURFACE EVERY 4 FEET IN ALL DIRECTIONS. STAKES MAY BE DRIVEN BEFORE OR AFTER APPLYING MULCH. SECURE MULCH TO SOIL SURFACE BY STRETCHING TWINE BETWEEN PEGS IN A CRISSCROSS AND SQUARE PATTERN. SECURE TWINE AROUND EACH PEG WITH TWO OR MORE ROUND TURNS.
- MULCH NETTINGS. STAPLE PAPER, JUICE, COTTON, OR PLASTIC NETTINGS TO THE SOIL SURFACE. USE A DEGRADABLE NETTING IN AREAS TO BE MOWED.
- CRIMPER (MULCH ANCHORING COULTER TOOL). A TRACTOR-DRAWN IMPLEMENT, SOMEWHAT LIKE A DISC HARROW, ESPECIALLY DESIGNED TO PUSH OR CUT SOME OF THE BROADCAST LONG FIBER MULCH 3 TO 4 INCHES INTO THE SOIL SO AS TO ANCHOR IT AND LEAVE PART STANDING UPRIGHT. THIS TECHNIQUE IS LIMITED TO AREAS TRAVERSABLE BY A TRACTOR, WHICH MUST OPERATE ON THE CONTOUR OF SLOPES. STRAW MULCH RATE MUST BE 3 TONS PER ACRE. NO TACKIFYING OR ADHESIVE AGENT IS REQUIRED.

- LIQUID MULCH BINDERS. MAY BE USED TO ANCHOR SALT HAY, HAY OR STRAW MULCH.

- APPLICATIONS SHOULD BE HEAVIER AT EDGES WHERE WIND MAY CATCH THE MULCH, IN VALLEYS, AND AT CRESTS OF BANKS. THE REMAINDER OF THE AREA SHOULD BE UNIFORM IN APPEARANCE.
- USE OF THE FOLLOWING:

- ORGANIC AND VEGETABLE BASED BINDERS. NATURALLY OCCURRING, POWDER BASED HYDROPHILIC MATERIALS WHEN MIXED WITH WATER FORMULATES A GEL AND WHEN APPLIED TO MULCH UNDER SATISFACTORY CURING CONDITIONS WILL FORM MEMBRANED NETWORKS OF INSOLUBLE POLYMERS. THE VEGETABLE GEL SHALL BE PHYSIOLOGICALLY HARMLESS AND NOT RESULT IN A PHYTOXIC EFFECT OR IMPEDE GROWTH OF TURFGRASS. USE AT RATES AND WEATHER CONDITIONS AS RECOMMENDED BY THE MANUFACTURER TO ANCHOR MULCH MATERIALS. MANY NEW PRODUCTS ARE AVAILABLE, SOME OF WHICH MAY NEED FURTHER EVALUATION FOR USE IN THE STATE.

CONSTRUCTION SEQUENCE

- | | |
|--|---------|
| 1. INSTALL ALL SOIL EROSION AND SEDIMENT CONTROL DEVICES | 1 DAY |
| 2. SITE CLEARING & ROUGH GRADING | 1 WEEK |
| 3. CONSTRUCTION OF SITE IMPROVEMENTS | 2 WEEKS |
| 4. RESTORE ALL AREAS AS APPROPRIATE COMPLETION OF CONSTRUCTION | 1 WEEK |
| 5. REMOVE SOIL EROSION AND SEDIMENT CONTROL DEVICES | 1 DAY |

AREA OF DISTURBANCE = 0.052 ACRES

SOIL DE-COMPACTION AND TESTING REQUIREMENTS

SOIL COMPACTION TESTING REQUIREMENTS

- SUBGRADE SOILS PRIOR TO THE APPLICATION OF TOPSOIL (SEE PERMANENT SEEDING AND STABILIZATION NOTES FOR TOPSOIL REQUIREMENTS) SHALL BE FREE OF EXCESSIVE COMPACTION TO A DEPTH OF 6.0 INCHES TO ENHANCE THE ESTABLISHMENT OF PERMANENT VEGETATIVE COVER.
- AREAS OF THE SITE WHICH ARE SUBJECT TO COMPACTION TESTING AND/OR MITIGATION ARE GRAPHICALLY DENOTED ON THE CERTIFIED SOIL EROSION CONTROL PLAN.
- COMPACTION TESTING LOCATIONS ARE DENOTED ON THE PLAN. A COPY OF THE PLAN OR PORTION OF THE PLAN SHALL BE USED TO MARK LOCATIONS OF TESTS, AND ATTACHED TO THE COMPACTION REMEDIATION FORM. AVAILABLE FROM THE LOCAL SOIL CONSERVATION DISTRICT. THIS FORM MUST BE FILLED OUT AND SUBMITTED PRIOR TO RECEIVING A CERTIFICATE OF COMPLIANCE FROM THE DISTRICT.
- IN THE EVENT THAT TESTING INDICATES COMPACTION IN EXCESS OF THE MAXIMUM THRESHOLDS INDICATED FOR THE SIMPLIFIED TESTING METHODS (SEE DETAILS BELOW), THE CONTRACTOR/OWNER SHALL HAVE THE OPTION TO PERFORM EITHER (1) COMPACTION MITIGATION OVER THE ENTIRE MITIGATION AREA DENOTED ON THE PLAN (EXCLUDING EXEMPT AREAS), OR (2) PERFORM ADDITIONAL MORE DETAILED TESTING TO ESTABLISH THE LIMITS OF EXCESSIVE COMPACTION WHEREUPON ONLY THE EXCESSIVELY COMPACTED AREAS WOULD REQUIRE COMPACTION MITIGATION. ADDITIONAL DETAILED TESTING SHALL BE PERFORMED BY A TRAINED, LICENSED PROFESSIONAL.

COMPACTION TESTING METHODS

- PROBING WIRE TEST (SEE DETAIL)
- HAND-HELD PENETROMETER TEST (SEE DETAIL)
- TUBE BULK DENSITY TEST (LICENSED PROFESSIONAL ENGINEER REQUIRED)
- NUCLEAR DENSITY TEST (LICENSED PROFESSIONAL ENGINEER REQUIRED)

NOTE: ADDITIONAL TESTING METHODS WHICH CONFORM TO ASTM STANDARDS AND SPECIFICATIONS, AND WHICH PRODUCE A DRY WEIGHT, SOIL BULK DENSITY MEASUREMENT MAY BE ALLOWED SUBJECT TO DISTRICT APPROVAL.

SOIL COMPACTION TESTING IS NOT REQUIRED IF/WHEN SUBSOIL COMPACTION REMEDIATION (SCARIFICATION/TILLAGE (6" MINIMUM DEPTH) OR SIMILAR) IS PROPOSED AS PART OF THE SEQUENCE OF CONSTRUCTION.

PROCEDURES FOR SOIL COMPACTION MITIGATION

PROCEDURES SHALL BE USED TO MITIGATE EXCESSIVE SOIL COMPACTION PRIOR TO PLACEMENT OF TOPSOIL AND ESTABLISHMENT OF PERMANENT VEGETATIVE COVER.

RESTORATION OF COMPACTED SOILS SHALL BE THROUGH DEEP SCARIFICATION/TILLAGE (6" MINIMUM DEPTH) WHERE THERE IS NO DANGER TO UNDERGROUND UTILITIES (CABLES, IRRIGATION SYSTEMS, ETC.). IN THE ALTERNATIVE, ANOTHER METHOD AS SPECIFIED BY A NEW JERSEY LICENSED PROFESSIONAL ENGINEER MAYBE SUBSTITUTED SUBJECT TO DISTRICT APPROVAL.

NOTES:

- THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY APPROVAL. THIS SET OF PLANS SHALL NOT BE UTILIZED FOR CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH DRAWING HAS BEEN MARKED "ISSUED FOR CONSTRUCTION."

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 A PROFESSIONAL ASSOCIATION
 CERTIFICATE OF AUTHORIZATION NO.: 24GA28021500 EXP. 8/31/2020

NO.	REVISION	BY	DATE
1	REV. PER BOROUGH COMMENTS	EM	2/4/2020

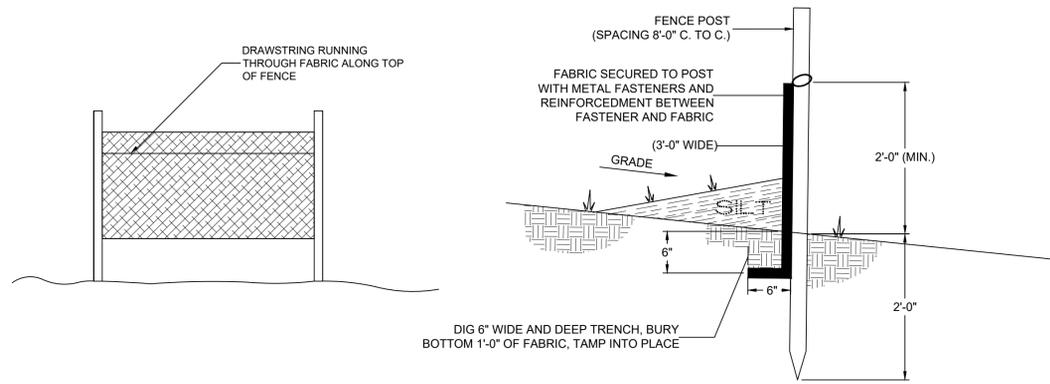
2/4/2020 DATE
 JOHN HANSEN
 PROFESSIONAL ENGINEER
 N.J. P.E. NO. 24GE04194500

PROJECT:
 BLACK RIVER
 VETERINARY HOSPITAL
 SITE PLANS
 BLOCK 103, LOT 51
 CHESTER BOROUGH
 MORRIS COUNTY NEW JERSEY

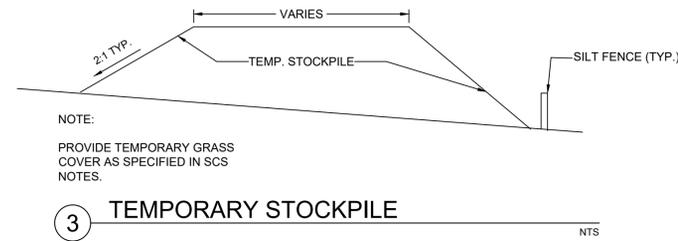
SOIL EROSION & SEDIMENT CONTROL NOTES & DETAILS

JOB NO.:	19003	DRAWING NO.:	8
SCALE:	AS SHOWN		
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	DETAILS.DWG		
DATE:	05/01/2019		10

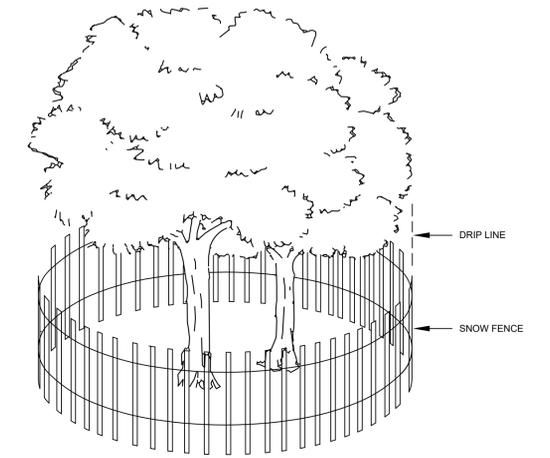
1 COMPACTION TESTING DETAIL



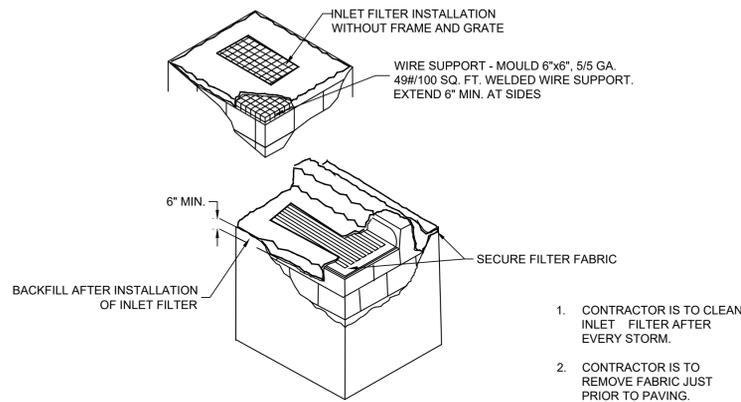
2 SILT FENCE DETAIL NTS



3 TEMPORARY STOCKPILE NTS

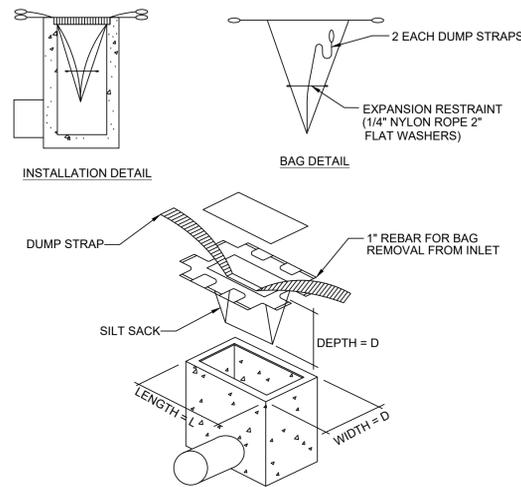


4 TREE PROTECTION DETAIL NTS

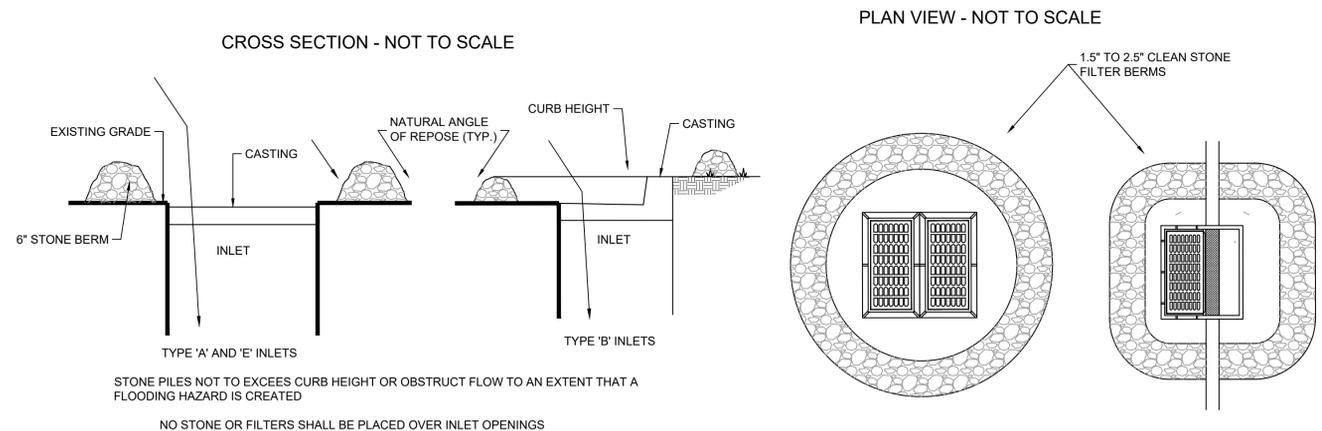


1. CONTRACTOR IS TO CLEAN INLET FILTER AFTER EVERY STORM.
2. CONTRACTOR IS TO REMOVE FABRIC JUST PRIOR TO PAVING.

5 INLET FILTER DETAIL NTS



6 SILT SACK NTS



7 INLET PROTECTION DETAIL NTS

NOTES:
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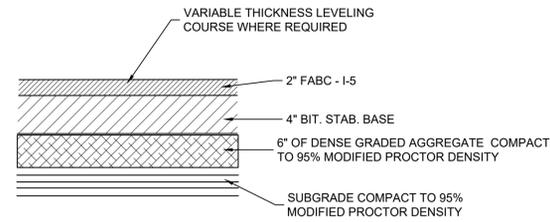
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NO.	REVISION	BY	DATE

2/4/2020
 DATE JOHN HANSEN
 PROFESSIONAL ENGINEER
 N.J. P.E. NO. 24GE04194500

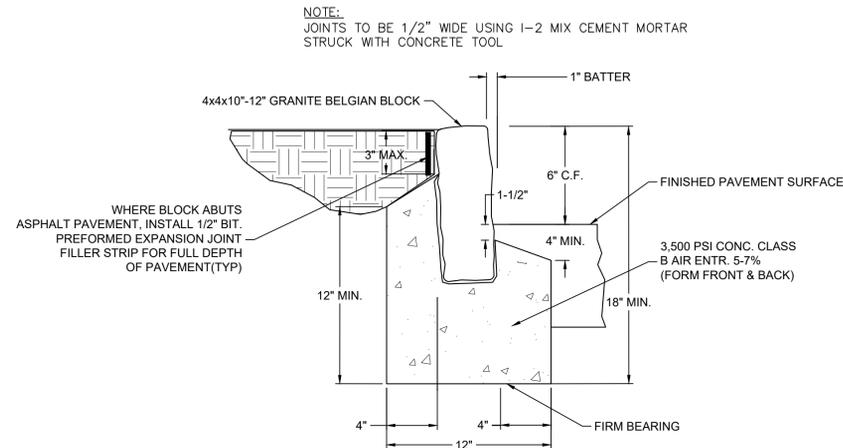
PROJECT:
 BLACK RIVER
 VETERINARY HOSPITAL
 SITE PLANS
 BLOCK 103, LOT 51
 CHESTER BOROUGH
 MORRIS COUNTY NEW JERSEY

TITLE:
**SOIL EROSION &
 SEDIMENT CONTROL
 DETAILS**

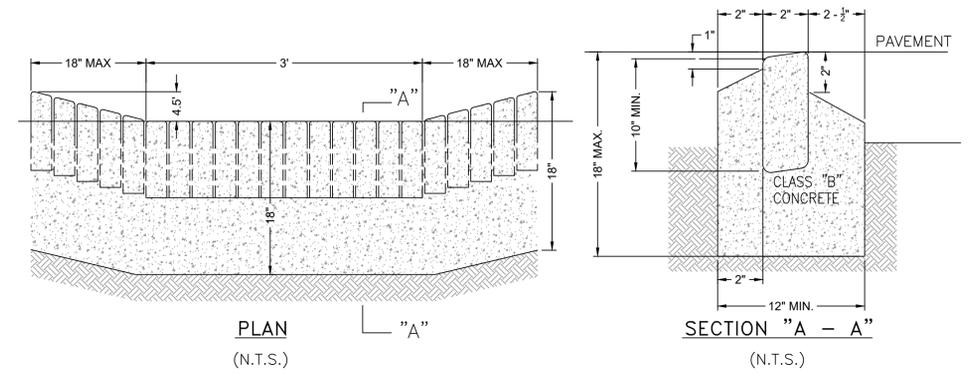
JOB NO.:	19003	9 10
SCALE:	AS SHOWN	
DESIGNED:	EM	
CHECKED:	JH	
FILENAME:	DETAILS.DWG	
DATE:	05/01/2019	



1 BITUMINOUS PAVING DETAIL
NTS

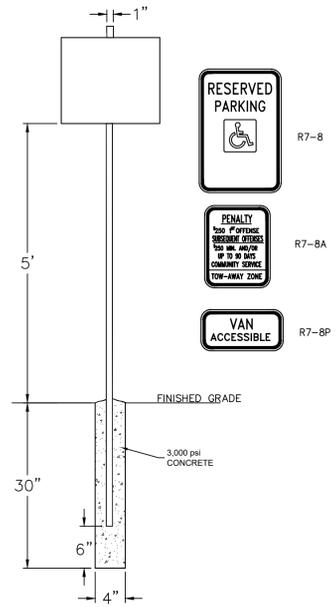


2 BELGIAN BLOCK GRANITE CURB DETAIL
NTS

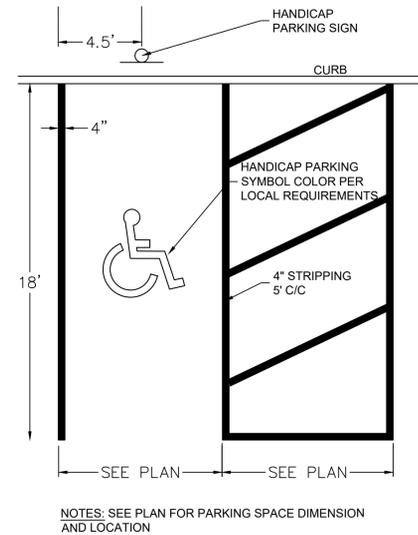


3 FLUSH GRANITE BLOCK CURB DETAIL
NTS
* CURB TO BE FLUSH WITH PARKING LOT AT ALL ACCESSIBLE ROUTES

NOTE:
DETAIL SHOWN PER PREVIOUS APPROVED SITE PLANS ENTITLED "PRELIMINARY AND FINAL SITE PLANS FOR LOT 51 BLOCK 103 'BLACK RIVER VETERINARY HOSPITAL' SITUATED IN THE BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY". PREPARED BY CIVIL ENGINEERING, INC., DATED 7/6/12 AND LAST REVISED 12/30/12.

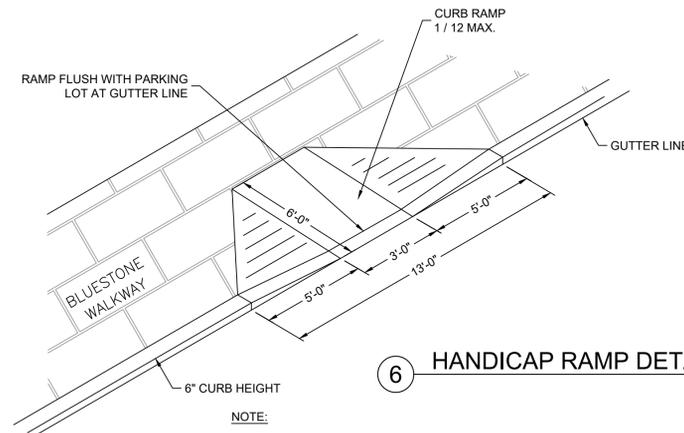


4 ADA PARKING SIGN DETAIL
NTS



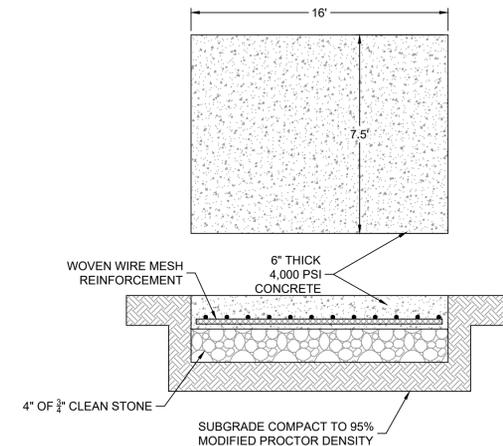
5 ADA PARKING STALL DETAIL
NTS

NOTES: SEE PLAN FOR PARKING SPACE DIMENSION AND LOCATION



6 HANDICAP RAMP DETAIL
NTS

NOTE:
DETAIL SHOWN PER PREVIOUS APPROVED SITE PLANS ENTITLED "PRELIMINARY AND FINAL SITE PLANS FOR LOT 51 BLOCK 103 'BLACK RIVER VETERINARY HOSPITAL' SITUATED IN THE BOROUGH OF CHESTER, MORRIS COUNTY, NEW JERSEY". PREPARED BY CIVIL ENGINEERING, INC., DATED 7/6/12 AND LAST REVISED 12/30/12.



7 CONCRETE PAD FOR REFUSE AREA ENCLOSURE
NTS

NOTES:
1. THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY APPROVAL. THIS SET OF PLANS SHALL NOT BE UTILIZED FOR CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH DRAWING HAS BEEN MARKED "ISSUED FOR CONSTRUCTION".

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140 WEST MAIN STREET HIGH BRIDGE, NJ 08829
PH. 908-238-0544 FAX. 908-238-9572
A PROFESSIONAL ASSOCIATION
CERTIFICATE OF AUTHORIZATION NO.: 24GA28021500 EXP. 8/31/2020

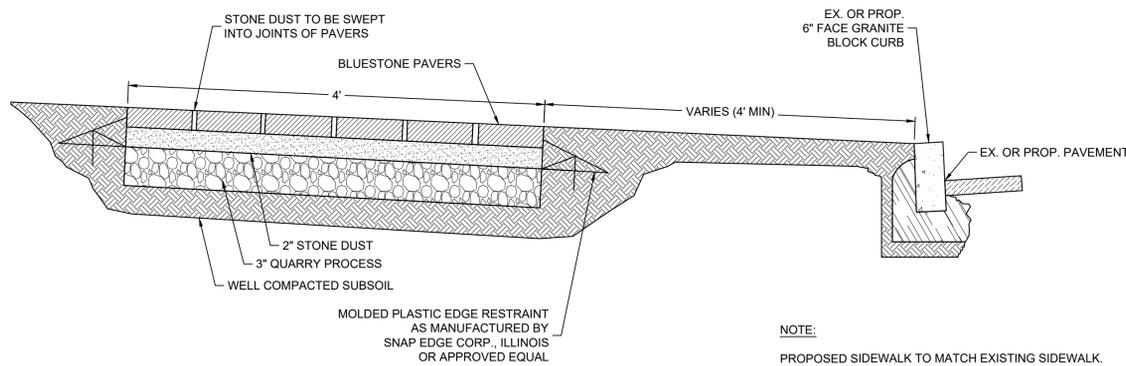
NO.	REVISION	BY	DATE
1	REV. PER BOROUGH COMMENTS	EM	2/4/2020

2/4/2020 DATE
JOHN HANSEN
PROFESSIONAL ENGINEER
N.J. P.E. NO. 24GE04194500

PROJECT:
BLACK RIVER VETERINARY HOSPITAL SITE PLANS
BLOCK 103, LOT 51
CHESTER BOROUGH
MORRIS COUNTY NEW JERSEY

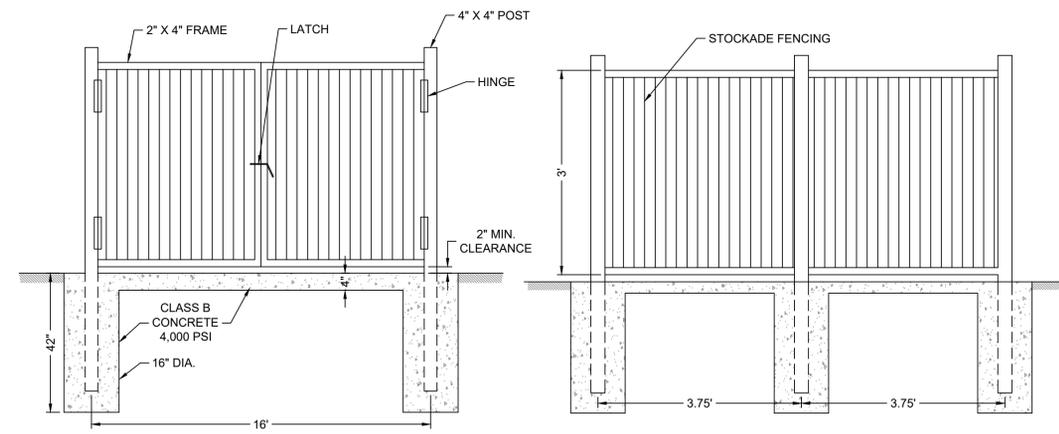
TITLE:
CONSTRUCTION DETAILS

JOB NO.:	19003	DRAWING NO.:	10
SCALE:	AS SHOWN		
DESIGNED:	EM		
CHECKED:	JH		
FILENAME:	DETAILS.DWG		
DATE:	05/01/2019		10



8 BLUESTONE PAVER SIDEWALK DETAIL
NTS

NOTE:
PROPOSED SIDEWALK TO MATCH EXISTING SIDEWALK.



9 TRASH ENCLOSURE DETAIL
NTS

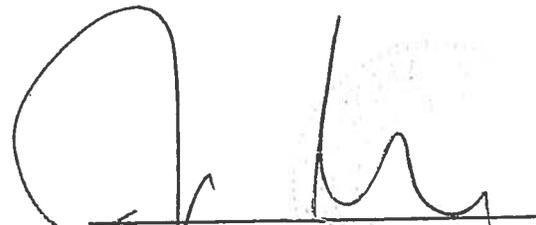
STORMWATER MANAGEMENT REPORT

**FOR
"BLACK RIVER VETERINARY HOSPITAL"**

**Lot 51 Block 103
Borough of Chester
Morris County, NJ**

PREPARED BY:

**CIVIL ENGINEERING INC.
1 COVE STREET
BUDD LAKE, NEW JERSEY
(973) 426-1776
(973) 426-0716 (FAX)**



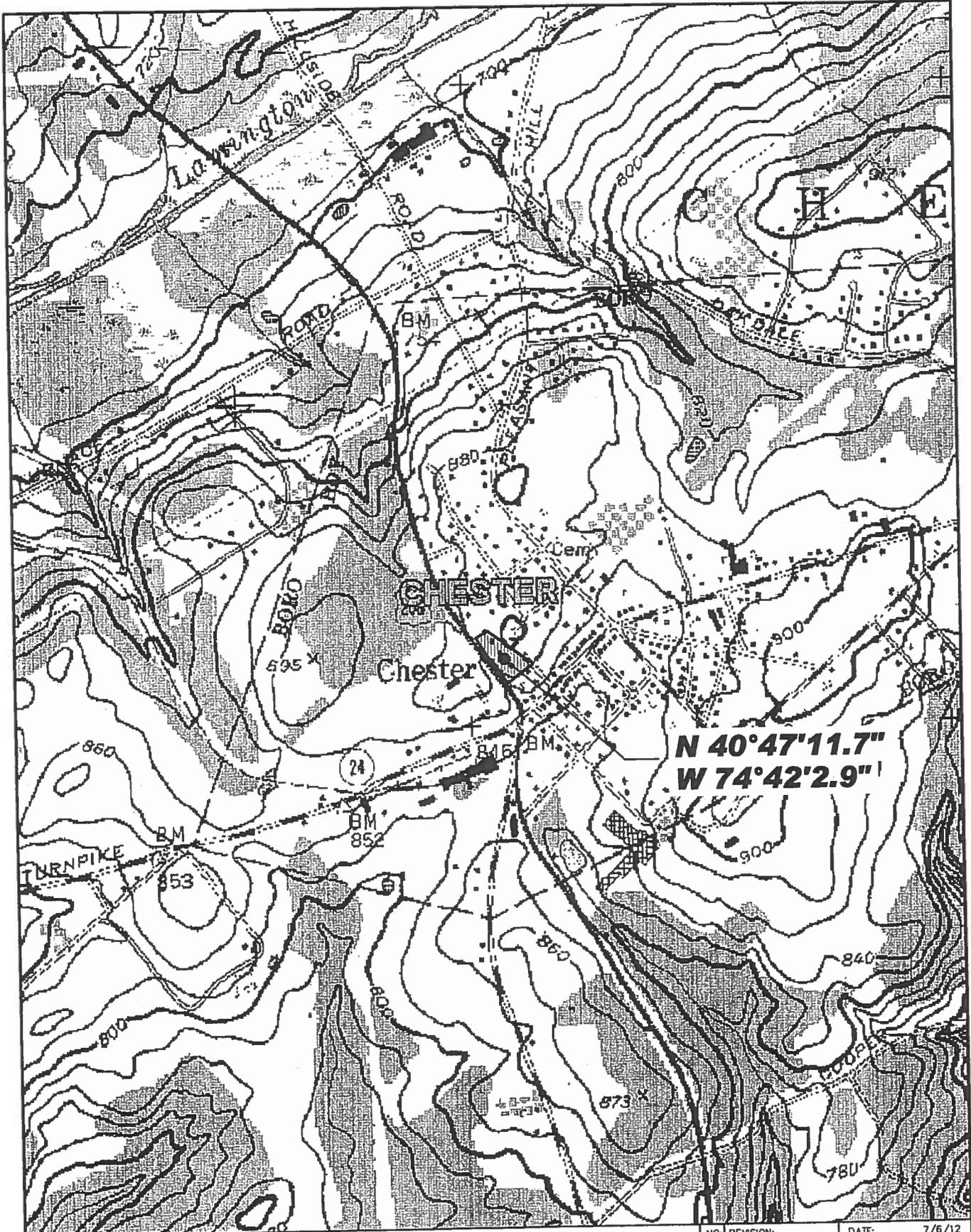
**JAMES GLASSON, P.E.
PROFESSIONAL ENGINEER
NJPE 87703**

July 06, 2012

Revised November 30, 2012

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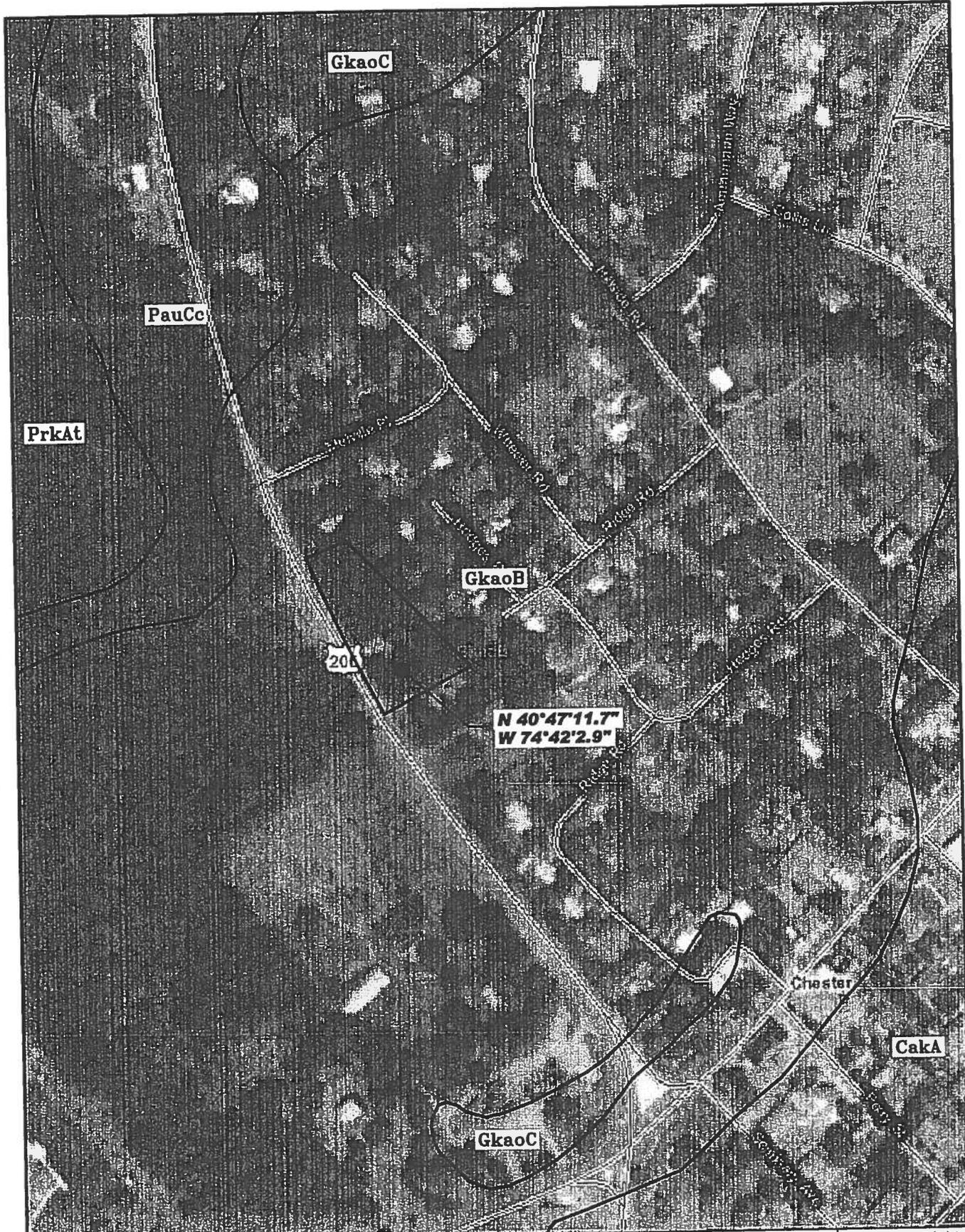
N 40°47'11.7"
W 74°42'2.9"

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 N.J. - C of A #24GA27922000

TACK SITE PLAN
 CHESTER
USGS QUADRANGLE MAP
 CHESTER BOROUGH
 MORRIS COUNTY, NEW JERSEY

NO:	REVISION:	DATE:	7/6/12
		SHEET:	1 OF 1
		SCALE:	N.T.S.
		PROJECT NO:	4923
		CHECKED BY:	JG
		DRAWN BY:	NBB



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 Fax: (973) 426-0716

N.J. - C of A #24GA27922000

TACK SITE PLAN
 CHESTER
SOIL CLASSIFICATION MAP

CHESTER BOROUGH
 MORRIS COUNTY, NEW JERSEY

NO:	REVISION:	DATE:	7/6/12
		SHEET:	1 OF 1
		SCALE:	N.T.S.
		PROJECT NO:	4923
		CHECKED BY:	JG
		DRAWN BY:	NBB

SUMMARY

The subject property is known as Lot 51 Block 103 as shown on Sheet 15 of the Borough of Chester Tax Maps. The property contains 1.19 Acres and is located along the northbound side of Route 206 (street address – 114 Route 206).

The site is currently the home of the "Black River Veterinary Hospital" which is comprised of a 1 story 1201 sf. framed building as well as a 7347 sf. asphalt and gravel parking area. The business is served by an on-lot cesspool for sewerage and currently has water supplied by from a New Jersey American owned main within Hedges Road. The property currently does not contain a storm drainage system although there is an 18 inch pipe within a low point on the lot which drains the site to the NJDOT Route 206 Drainage System.

The proposal is to renovate the building and add a 2589 sf. addition to create a new 3790 sf. one story Animal Hospital Building. A 25 space paved and curbed parking lot (10,362 sf.) is also proposed with a designated loading area.

By definition, the project is not a "major development"

- Less than ¼ acre of additional coverage is proposed
- Disturbance is less than 1 acre

This means that the project does not require compliance with the NJ Best Management Practices (BMP) guidelines.

We have analyzed the runoff peak flows based upon the Rational Method while the storage required has been based upon the Modified Rational Method.

Pre Development

Pre Development conditions encompass the area of the entire property (1.19 acres designated as A on the attached Pre Development Drainage Area Plan) which flows to an existing open 18 inch pipe which crosses Route 206.

Pre Development Area A – Peak Flows(1.19 Ac.)

Q ₁₀₀	=	3.49 CFS
Q ₁₀	=	2.50 CFS
Q ₂	=	1.84 CFS

Post Development

Post Development Conditions are broken down into 2 areas, Remainder Area A & Area B. Area A encompasses the area of the site (.78 ac.) which continues to flow undetained to the existing 18 inch pipe which crosses Route 206.

Post Development Area A – Peak Flows (.78 Ac.)

Q ₁₀₀	=	1.84 CFS
Q ₁₀	=	1.32 CFS
Q ₂	=	.97 CFS

Area B encompasses the area of the site (.41 ac.) which contains the improvements including the building (with addition), the new parking lot, etc. which is directed to the on-lot drywell infiltration system.

Post Development Area B – Peak Flows (.41 Ac.)

Q₁₀₀ = 2.92 CFS
 Q₁₀ = 2.12 CFS
 Q₂ = 1.53 CFS

In order to calculate the volume of storage required to infiltrate the 100 YR runoff generated to Area B, we have utilized the Modified Rational Method and analyzed the storage required based upon required IDF Curve 100 YR intensities. The maximum storage calculated is 4230 CF of volume.

To infiltrate this volume, we have design a drywell infiltration system with direct piping from the new roof area and storm drainage piping from the parking areas. The last inlet within the parking lot storm system, prior to the infiltration system, has been equipped with a sump area and hood to eliminate any chance of debris entering the system (details are shown within the Site Plans).

Per NJAC 7:8-5.4(a)3ii, the Peak Post Development Flow offsite to the storm system on Route 206 must be reduced to state factored pre development rates:

Pre Development Area A

Pre Q₁₀₀ = 3.49 CFS @ 80% = 2.79 CFS Allowed
 Q₁₀ = 2.50 CFS @ 75% = 1.875 CFS Allowed
 Q₂ = 1.84 CFS @ 50% = .92 CFS Allowed

Remainder Area A

Post Q₁₀₀ = 1.84 CFS Proposed
 Q₁₀ = 1.32 CFS Proposed
 Q₂ = .91 CFS Proposed

The Post Development Peak Flows offsite (Remainder Area A) have been reduced per state requirements for the 100 year, 10 year and 2 year storms.

Soil Logs and soils tests were performed in the area of the proposed infiltration system on site and the results are attached. The logs and testing prove that the site has adequate soil to provide required infiltration of the runoff and the system can drain within the allotted time period allowed by state requirements (shown within report).

This report should be reviewed in conjunction with the Pre and Post Development Drainage Area Plans attached to the report and also shown within the Site Plans.

PRE DEVELOPMENT AREA A

Pre Development Area A encompasses the entire site (1.19 Ac.) which flows to an open 18 inch storm pipe along the west property line/Route 206 Right of Way line.

$Q = CiA$

I. C = Runoff Coefficient

C =	<u>Area</u>	<u>Condition</u>	<u>Coefficient</u>
	.17 ac.	Pavement, Conc., Roof	.99
	.04 ac.	Gravel	.75
	.52 ac.	Grass	.45
	.46 ac.	woods	.25
	<u>1.19</u>	$C_{AVG} = .46$	

II. i=Intensity = Function of time of Concentration

$T_c = 16.2$ Minutes (see next page)

III. A = Area = ~~1.41~~ ^{1.19} Ac.

$Q_{100} = \cancel{6.54}$ CFS ~ 3.59
 $Q_{10} = \cancel{4.74}$ CFS ~ 2.57
 $Q_2 = \cancel{3.51}$ CFS ~ 1.89

Project LOT 51 BCK 10.3 , 16
 Location CHESTER BOROUGH Checked _____ Date 7/3/12
 Circle one: Present Developed
 Circle one: T_c T_c through subarea PRE DEV. AREA A

NOTES: Space for as many as two segments per flow type can be used for each worksheet.
 Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to T_c only)

- Segment ID
- Surface description (Table 3-1)
 - Manning's roughness coeff., n (Table 3-1) ..
 - Flow length, L (total L ≤ 300 ft) ft
 - Two-yr 24-hr rainfall, P₂ in
 - Land slope, s ft/ft
 - $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$, 123/50 Compute T_c hr

AB	
GRASS	
.24	
150	
3.3	
4.196	
.25	

= 15 MIN

Shallow concentrated flow

- Segment ID
- Surface description (paved or unpaved)
 - Flow length, L ft
 - Watercourse slope, s ft/ft
 - Average velocity, V (Figure 3-1) ft/s
 - $T_c = \frac{L}{3600 V}$ Compute T_c hr

BC	
UNPAVED	
210	
3.6	
3.0	
.019	

= 1.2 MIN

Channel flow

- Segment ID
- Cross sectional flow area, A ft²
 - Wetted perimeter, P_w ft
 - Hydraulic radius, $r = \frac{A}{P_w}$ Compute r ft
 - Channel slope, s ft/ft
 - Manning's roughness coeff., n
 - $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ft/s
 - Flow length, L ft
 - $T_c = \frac{L}{3600 V}$ Compute T_c hr
 - Water shed or subarea T_c or T_c (add T_c in steps 6, 11, and 19) hr

= 16.2 MIN

Hydrograph for Subcatchment 1S: Pre Dev. Area A

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.27	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.54	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.80	0.98	0.00	1.90	0.00	2.82	0.00
0.08	1.07	1.00	0.00	1.92	0.00	2.84	0.00
0.10	1.34	1.02	0.00	1.94	0.00	2.86	0.00
0.12	1.61	1.04	0.00	1.96	0.00	2.88	0.00
0.14	1.88	1.06	0.00	1.98	0.00	2.90	0.00
0.16	2.15	1.08	0.00	2.00	0.00	2.92	0.00
0.18	2.41	1.10	0.00	2.02	0.00	2.94	0.00
0.20	2.68	1.12	0.00	2.04	0.00	2.96	0.00
0.22	2.95	1.14	0.00	2.06	0.00	2.98	0.00
0.24	3.22	1.16	0.00	2.08	0.00	3.00	0.00
0.26	3.49	1.18	0.00	2.10	0.00		
0.28	3.40	1.20	0.00	2.12	0.00		
0.30	3.13	1.22	0.00	2.14	0.00		
0.32	2.86	1.24	0.00	2.16	0.00		
0.34	2.59	1.26	0.00	2.18	0.00		
0.36	2.33	1.28	0.00	2.20	0.00		
0.38	2.06	1.30	0.00	2.22	0.00		
0.40	1.79	1.32	0.00	2.24	0.00		
0.42	1.52	1.34	0.00	2.26	0.00		
0.44	1.25	1.36	0.00	2.28	0.00		
0.46	0.98	1.38	0.00	2.30	0.00		
0.48	0.72	1.40	0.00	2.32	0.00		
0.50	0.45	1.42	0.00	2.34	0.00		
0.52	0.18	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.58	0.00	2.52	0.00		
0.70	0.00	1.60	0.00	2.54	0.00		
0.72	0.00	1.62	0.00	2.56	0.00		
0.74	0.00	1.64	0.00	2.58	0.00		
0.76	0.00	1.66	0.00	2.60	0.00		
0.78	0.00	1.68	0.00	2.62	0.00		
0.80	0.00	1.70	0.00	2.64	0.00		
0.82	0.00	1.72	0.00	2.66	0.00		
0.84	0.00	1.74	0.00	2.68	0.00		
0.86	0.00	1.76	0.00	2.70	0.00		
0.88	0.00	1.78	0.00	2.72	0.00		
0.90	0.00	1.80	0.00	2.74	0.00		
		1.82	0.00				

Hydrograph for Subcatchment 1S: Pre Dev. Area A

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.19	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.38	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.58	0.98	0.00	1.90	0.00	2.82	0.00
0.08	0.77	1.00	0.00	1.92	0.00	2.84	0.00
0.10	0.96	1.02	0.00	1.94	0.00	2.86	0.00
0.12	1.15	1.04	0.00	1.96	0.00	2.88	0.00
0.14	1.35	1.06	0.00	1.98	0.00	2.90	0.00
0.16	1.54	1.08	0.00	2.00	0.00	2.92	0.00
0.18	1.73	1.10	0.00	2.02	0.00	2.94	0.00
0.20	1.92	1.12	0.00	2.04	0.00	2.96	0.00
0.22	2.12	1.14	0.00	2.06	0.00	2.98	0.00
0.24	2.31	1.16	0.00	2.08	0.00	3.00	0.00
0.26	2.50	1.18	0.00	2.10	0.00		
0.28	2.44	1.20	0.00	2.12	0.00		
0.30	2.25	1.22	0.00	2.14	0.00		
0.32	2.05	1.24	0.00	2.16	0.00		
0.34	1.86	1.26	0.00	2.18	0.00		
0.36	1.67	1.28	0.00	2.20	0.00		
0.38	1.48	1.30	0.00	2.22	0.00		
0.40	1.28	1.32	0.00	2.24	0.00		
0.42	1.09	1.34	0.00	2.26	0.00		
0.44	0.90	1.36	0.00	2.28	0.00		
0.46	0.71	1.38	0.00	2.30	0.00		
0.48	0.51	1.40	0.00	2.32	0.00		
0.50	0.32	1.42	0.00	2.34	0.00		
0.52	0.13	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

Hydrograph for Subcatchment 1S: Pre Dev. Area A

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.14	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.28	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.42	0.98	0.00	1.90	0.00	2.82	0.00
0.08	0.57	1.00	0.00	1.92	0.00	2.84	0.00
0.10	0.71	1.02	0.00	1.94	0.00	2.86	0.00
0.12	0.85	1.04	0.00	1.96	0.00	2.88	0.00
0.14	0.99	1.06	0.00	1.98	0.00	2.90	0.00
0.16	1.13	1.08	0.00	2.00	0.00	2.92	0.00
0.18	1.27	1.10	0.00	2.02	0.00	2.94	0.00
0.20	1.41	1.12	0.00	2.04	0.00	2.96	0.00
0.22	1.55	1.14	0.00	2.06	0.00	2.98	0.00
0.24	1.70	1.16	0.00	2.08	0.00	3.00	0.00
0.26	1.84	1.18	0.00	2.10	0.00		
0.28	1.79	1.20	0.00	2.12	0.00		
0.30	1.65	1.22	0.00	2.14	0.00		
0.32	1.51	1.24	0.00	2.16	0.00		
0.34	1.37	1.26	0.00	2.18	0.00		
0.36	1.22	1.28	0.00	2.20	0.00		
0.38	1.08	1.30	0.00	2.22	0.00		
0.40	0.94	1.32	0.00	2.24	0.00		
0.42	0.80	1.34	0.00	2.26	0.00		
0.44	0.66	1.36	0.00	2.28	0.00		
0.46	0.52	1.38	0.00	2.30	0.00		
0.48	0.38	1.40	0.00	2.32	0.00		
0.50	0.24	1.42	0.00	2.34	0.00		
0.52	0.09	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

POST DEVELOPMENT REMAINDER AREA A

Post Development Area A encompasses the remaining Area (.78 Acres) of the site that continues to flow undetained to the open 18 inch storm pipe along the west property line/Route 206 Right of Way line.

$$Q = CiA$$

I. C = Runoff Coefficient

C =	<u>Area</u>	<u>Condition</u>	<u>Coefficient</u>
	.44 ac.	Grass	.45
	.33 ac.	woods	.25
	.01	Conc.	.99

$$C_{AVG} = .37$$

II. i=Intensity = Function of time of Concentration

$$T_c = 16.2 \text{ Minutes (see next page)}$$

III. A = Area = .78 ac.

Hydrograph for Subcatchment 2S: Post Dev. Area A

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.14	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.28	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.42	0.98	0.00	1.90	0.00	2.82	0.00
0.08	0.57	1.00	0.00	1.92	0.00	2.84	0.00
0.10	0.71	1.02	0.00	1.94	0.00	2.86	0.00
0.12	0.85	1.04	0.00	1.96	0.00	2.88	0.00
0.14	0.99	1.06	0.00	1.98	0.00	2.90	0.00
0.16	1.13	1.08	0.00	2.00	0.00	2.92	0.00
0.18	1.27	1.10	0.00	2.02	0.00	2.94	0.00
0.20	1.41	1.12	0.00	2.04	0.00	2.96	0.00
0.22	1.56	1.14	0.00	2.06	0.00	2.98	0.00
0.24	1.70	1.16	0.00	2.08	0.00	3.00	0.00
0.26	1.84	1.18	0.00	2.10	0.00		
0.28	1.79	1.20	0.00	2.12	0.00		
0.30	1.65	1.22	0.00	2.14	0.00		
0.32	1.51	1.24	0.00	2.16	0.00		
0.34	1.37	1.26	0.00	2.18	0.00		
0.36	1.23	1.28	0.00	2.20	0.00		
0.38	1.08	1.30	0.00	2.22	0.00		
0.40	0.94	1.32	0.00	2.24	0.00		
0.42	0.80	1.34	0.00	2.26	0.00		
0.44	0.66	1.36	0.00	2.28	0.00		
0.46	0.52	1.38	0.00	2.30	0.00		
0.48	0.38	1.40	0.00	2.32	0.00		
0.50	0.24	1.42	0.00	2.34	0.00		
0.52	0.09	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

Hydrograph for Subcatchment 2S: Post Dev. Area A

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.10	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.20	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.30	0.98	0.00	1.90	0.00	2.82	0.00
0.08	0.41	1.00	0.00	1.92	0.00	2.84	0.00
0.10	0.51	1.02	0.00	1.94	0.00	2.86	0.00
0.12	0.61	1.04	0.00	1.96	0.00	2.88	0.00
0.14	0.71	1.06	0.00	1.98	0.00	2.90	0.00
0.16	0.81	1.08	0.00	2.00	0.00	2.92	0.00
0.18	0.91	1.10	0.00	2.02	0.00	2.94	0.00
0.20	1.01	1.12	0.00	2.04	0.00	2.96	0.00
0.22	1.12	1.14	0.00	2.06	0.00	2.98	0.00
0.24	1.22	1.16	0.00	2.08	0.00	3.00	0.00
0.26	1.32	1.18	0.00	2.10	0.00		
0.28	1.29	1.20	0.00	2.12	0.00		
0.30	1.18	1.22	0.00	2.14	0.00		
0.32	1.08	1.24	0.00	2.16	0.00		
0.34	0.98	1.26	0.00	2.18	0.00		
0.36	0.88	1.28	0.00	2.20	0.00		
0.38	0.78	1.30	0.00	2.22	0.00		
0.40	0.68	1.32	0.00	2.24	0.00		
0.42	0.58	1.34	0.00	2.26	0.00		
0.44	0.47	1.36	0.00	2.28	0.00		
0.46	0.37	1.38	0.00	2.30	0.00		
0.48	0.27	1.40	0.00	2.32	0.00		
0.50	0.17	1.42	0.00	2.34	0.00		
0.52	0.07	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

Hydrograph for Subcatchment 2S: Post Dev. Area A

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.06	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.12	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.18	0.98	0.00	1.90	0.00	2.82	0.00
0.08	0.24	1.00	0.00	1.92	0.00	2.84	0.00
0.10	0.30	1.02	0.00	1.94	0.00	2.86	0.00
0.12	0.36	1.04	0.00	1.96	0.00	2.88	0.00
0.14	0.42	1.06	0.00	1.98	0.00	2.90	0.00
0.16	0.48	1.08	0.00	2.00	0.00	2.92	0.00
0.18	0.54	1.10	0.00	2.02	0.00	2.94	0.00
0.20	0.61	1.12	0.00	2.04	0.00	2.96	0.00
0.22	0.67	1.14	0.00	2.06	0.00	2.98	0.00
0.24	0.73	1.16	0.00	2.08	0.00	3.00	0.00
0.26	0.79	1.18	0.00	2.10	0.00		
0.28	0.85	1.20	0.00	2.12	0.00		
0.30	0.91	1.22	0.00	2.14	0.00		
0.32	0.85	1.24	0.00	2.16	0.00		
0.34	0.79	1.26	0.00	2.18	0.00		
0.36	0.73	1.28	0.00	2.20	0.00		
0.38	0.67	1.30	0.00	2.22	0.00		
0.40	0.61	1.32	0.00	2.24	0.00		
0.42	0.54	1.34	0.00	2.26	0.00		
0.44	0.48	1.36	0.00	2.28	0.00		
0.46	0.42	1.38	0.00	2.30	0.00		
0.48	0.36	1.40	0.00	2.32	0.00		
0.50	0.30	1.42	0.00	2.34	0.00		
0.52	0.24	1.44	0.00	2.36	0.00		
0.54	0.18	1.46	0.00	2.38	0.00		
0.56	0.12	1.48	0.00	2.40	0.00		
0.58	0.06	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

POST DEVELOPMENT AREA B

Post Development Area B encompasses .41 acres of area which contains the proposed site improvements including the building and entire parking area. These areas are directed to a proposed storm sewer system which flows to an underground drywell infiltration system.

$$Q = CiA$$

I. C = Runoff Coefficient

C =	<u>Area</u>	<u>Condition</u>	<u>Coefficient</u>
	.05 ac.	Grass	.45
	.36 ac.	Roof, Pavement, Conc.	.99

$$C_{AVG} = .92$$

II. i=Intensity = Function of time of Concentration

$$T_c = 10.25 \text{ Minutes (see Inlet Calculations)}$$

III. A = Area = .41 ac.

Hydrograph for Subcatchment 3S: Post Dev. Area B to Drywell Infiltration

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.37	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.73	0.96	0.00	1.88	0.00	2.80	0.00
0.06	1.10	0.98	0.00	1.90	0.00	2.82	0.00
0.08	1.46	1.00	0.00	1.92	0.00	2.84	0.00
0.10	1.83	1.02	0.00	1.94	0.00	2.86	0.00
0.12	2.19	1.04	0.00	1.96	0.00	2.88	0.00
0.14	2.56	1.06	0.00	1.98	0.00	2.90	0.00
0.16	2.92	1.08	0.00	2.00	0.00	2.92	0.00
0.18	2.80	1.10	0.00	2.02	0.00	2.94	0.00
0.20	2.43	1.12	0.00	2.04	0.00	2.96	0.00
0.22	2.07	1.14	0.00	2.06	0.00	2.98	0.00
0.24	1.70	1.16	0.00	2.08	0.00	3.00	0.00
0.26	1.34	1.18	0.00	2.10	0.00		
0.28	0.97	1.20	0.00	2.12	0.00		
0.30	0.61	1.22	0.00	2.14	0.00		
0.32	0.24	1.24	0.00	2.16	0.00		
0.34	0.00	1.26	0.00	2.18	0.00		
0.36	0.00	1.28	0.00	2.20	0.00		
0.38	0.00	1.30	0.00	2.22	0.00		
0.40	0.00	1.32	0.00	2.24	0.00		
0.42	0.00	1.34	0.00	2.26	0.00		
0.44	0.00	1.36	0.00	2.28	0.00		
0.46	0.00	1.38	0.00	2.30	0.00		
0.48	0.00	1.40	0.00	2.32	0.00		
0.50	0.00	1.42	0.00	2.34	0.00		
0.52	0.00	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

Hydrograph for Subcatchment 3S: Post Dev. Area B to Drywell Infiltration

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.26	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.53	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.79	0.98	0.00	1.90	0.00	2.82	0.00
0.08	1.06	1.00	0.00	1.92	0.00	2.84	0.00
0.10	1.32	1.02	0.00	1.94	0.00	2.86	0.00
0.12	1.59	1.04	0.00	1.96	0.00	2.88	0.00
0.14	1.85	1.06	0.00	1.98	0.00	2.90	0.00
0.16	2.12	1.08	0.00	2.00	0.00	2.92	0.00
0.18	2.03	1.10	0.00	2.02	0.00	2.94	0.00
0.20	1.76	1.12	0.00	2.04	0.00	2.96	0.00
0.22	1.50	1.14	0.00	2.06	0.00	2.98	0.00
0.24	1.24	1.16	0.00	2.08	0.00	3.00	0.00
0.26	0.97	1.18	0.00	2.10	0.00		
0.28	0.71	1.20	0.00	2.12	0.00		
0.30	0.44	1.22	0.00	2.14	0.00		
0.32	0.18	1.24	0.00	2.16	0.00		
0.34	0.00	1.26	0.00	2.18	0.00		
0.36	0.00	1.28	0.00	2.20	0.00		
0.38	0.00	1.30	0.00	2.22	0.00		
0.40	0.00	1.32	0.00	2.24	0.00		
0.42	0.00	1.34	0.00	2.26	0.00		
0.44	0.00	1.36	0.00	2.28	0.00		
0.46	0.00	1.38	0.00	2.30	0.00		
0.48	0.00	1.40	0.00	2.32	0.00		
0.50	0.00	1.42	0.00	2.34	0.00		
0.52	0.00	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

Hydrograph for Subcatchment 3S: Post Dev. Area B to Drywell Infiltration

Time (hours)	Runoff (cfs)						
0.00	0.00	0.92	0.00	1.84	0.00	2.76	0.00
0.02	0.19	0.94	0.00	1.86	0.00	2.78	0.00
0.04	0.38	0.96	0.00	1.88	0.00	2.80	0.00
0.06	0.58	0.98	0.00	1.90	0.00	2.82	0.00
0.08	0.77	1.00	0.00	1.92	0.00	2.84	0.00
0.10	0.96	1.02	0.00	1.94	0.00	2.86	0.00
0.12	1.15	1.04	0.00	1.96	0.00	2.88	0.00
0.14	1.34	1.06	0.00	1.98	0.00	2.90	0.00
0.16	1.53	1.08	0.00	2.00	0.00	2.92	0.00
0.18	1.47	1.10	0.00	2.02	0.00	2.94	0.00
0.20	1.28	1.12	0.00	2.04	0.00	2.96	0.00
0.22	1.09	1.14	0.00	2.06	0.00	2.98	0.00
0.24	0.89	1.16	0.00	2.08	0.00	3.00	0.00
0.26	0.70	1.18	0.00	2.10	0.00		
0.28	0.51	1.20	0.00	2.12	0.00		
0.30	0.32	1.22	0.00	2.14	0.00		
0.32	0.13	1.24	0.00	2.16	0.00		
0.34	0.00	1.26	0.00	2.18	0.00		
0.36	0.00	1.28	0.00	2.20	0.00		
0.38	0.00	1.30	0.00	2.22	0.00		
0.40	0.00	1.32	0.00	2.24	0.00		
0.42	0.00	1.34	0.00	2.26	0.00		
0.44	0.00	1.36	0.00	2.28	0.00		
0.46	0.00	1.38	0.00	2.30	0.00		
0.48	0.00	1.40	0.00	2.32	0.00		
0.50	0.00	1.42	0.00	2.34	0.00		
0.52	0.00	1.44	0.00	2.36	0.00		
0.54	0.00	1.46	0.00	2.38	0.00		
0.56	0.00	1.48	0.00	2.40	0.00		
0.58	0.00	1.50	0.00	2.42	0.00		
0.60	0.00	1.52	0.00	2.44	0.00		
0.62	0.00	1.54	0.00	2.46	0.00		
0.64	0.00	1.56	0.00	2.48	0.00		
0.66	0.00	1.58	0.00	2.50	0.00		
0.68	0.00	1.60	0.00	2.52	0.00		
0.70	0.00	1.62	0.00	2.54	0.00		
0.72	0.00	1.64	0.00	2.56	0.00		
0.74	0.00	1.66	0.00	2.58	0.00		
0.76	0.00	1.68	0.00	2.60	0.00		
0.78	0.00	1.70	0.00	2.62	0.00		
0.80	0.00	1.72	0.00	2.64	0.00		
0.82	0.00	1.74	0.00	2.66	0.00		
0.84	0.00	1.76	0.00	2.68	0.00		
0.86	0.00	1.78	0.00	2.70	0.00		
0.88	0.00	1.80	0.00	2.72	0.00		
0.90	0.00	1.82	0.00	2.74	0.00		

STORAGE REQUIRED TO INFILTRATION SYSTEM

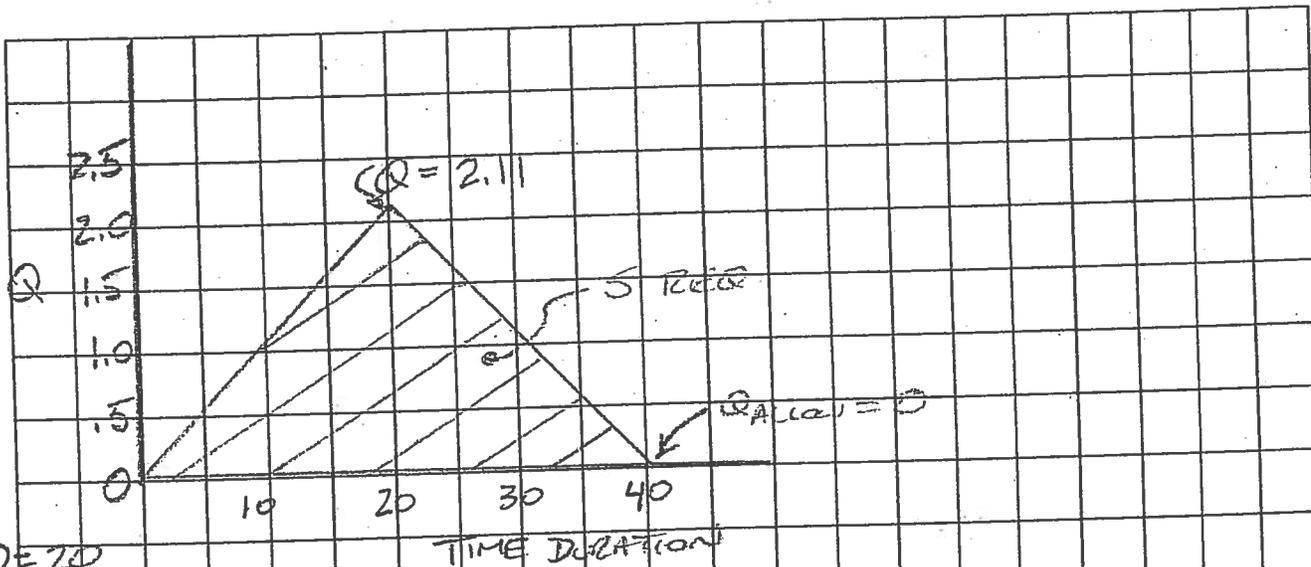
In order to determine the 100 Year volume of runoff generated by Area B, we have used the Modified Rational Method

Drainage Area .41 Ac.
 Runoff Coefficient .92
 Time of Concentration 10.25 Min.

<u>Storm Duration</u>	<u>IDF Curve 100 YR Intensity (in/Hr.)</u>	<u>C</u>	<u>A (Ac.)</u>	<u>Peak Runoff (CiA)</u>
20 min.	5.6	.92	.41	2.11 cfs.
30 min.	4.6	.92	.41	1.74 cfs.
45 min.	3.6	.92	.41	1.36 cfs.
60 min.	2.9	.92	.41	1.09 cfs.
90 min.	2.2	.92	.41	.83 cfs.
120 min.	1.7	.92	.41	.64 cfs.
180 min.	1.3	.92	.41	.49 cfs.
240	1.1	.92	.41	.41 cfs.

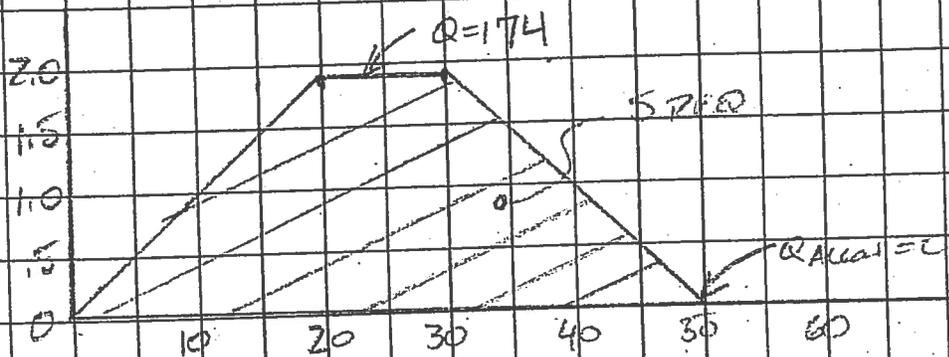
<u>Storm Duration</u>	<u>Q in</u>	<u>S in</u>	<u>Q (runoff allowable)</u>	<u>SA (storage allowable)</u>	<u>SR (storage req.)</u>
20 min.	2.11	2532	0	0	2532 cf.
30 min.	1.74	3132	0	0	3132 cf.
45 min.	1.36	4080	0	0	4080 cf.***
60 min.	1.09	3924	0	0	3924 cf.

***** 4080 cf. of storage required**



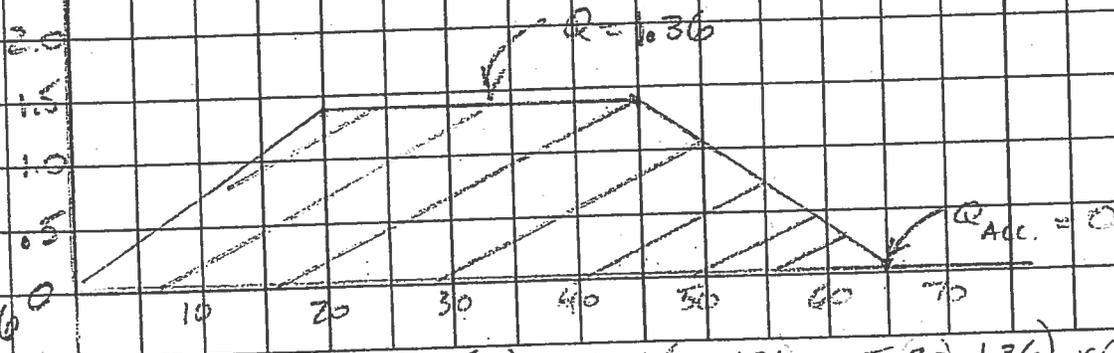
D = 20
Q = 2.11

$$S_{11} = (20(2.11)(.5)) \times 60 = 1532$$



D = 30
Q = 1.74

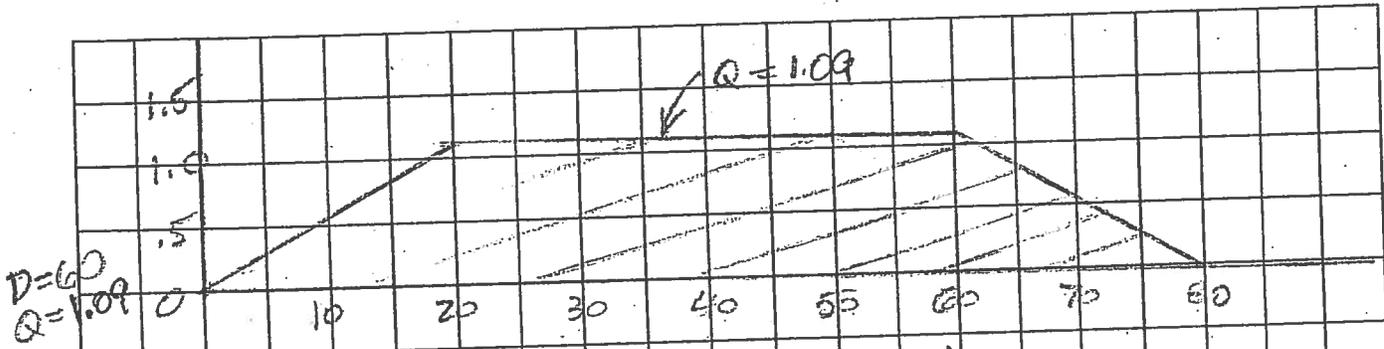
$$S_{11} = ((.5(20)(1.74)) + (10 \times 1.74) + (.5(20)(1.74))) \times 60 = 3132$$



D = 45
Q = 1.36

$$S_{11} = (.5(20)(1.36) + (30 \times 1.36) + .5(20)(1.36)) \times 60 = 4080$$

CIVIL ENGINEERING, INC.



$$\sum_{\text{IN}} \left(.5(20)1.09 + 40 \times 1.09 + .5(20)1.09 \right) (60) = 3924$$

INFILTRATION SYSTEM DESIGN

The soils onsite are Gladstone Gravelly Loams which are in hydrological soils group B. This group allows for drywell infiltration design for storms greater than the groundwater recharge storm event. The design permeability rate of the subgrade soils must be sufficient to fully drain the drywells maximum design storm runoff volume within 72 hours.

Based upon soil logs and soils testing performed on the site, the tested permeability rate of the soils is 10 minutes per inch or 6 inches per hour. Based upon a required factor of safety of two (2), the design rate would be 3 inches per hour.

Using this design rate, the drywell system would have to fully drain the drywells maximum design storm runoff (4080 cf.) within 72 hours. The drywell design requires nine 8 foot diameter drywells in a common excavation at a depth of 90 inches.

$$90 \text{ inches} / 3 \text{ inches/hr.} = 30 \text{ hours}$$

This time period of 30 hrs. is within the allowable 72 hour period.

Date: November 28, 2012

Project: Tack-Chester

Job Number: June 23, 1913

4080.00 Total Water (cu. Ft.)

DRYWELL CALCULATIONS

Assumptions

- 8.3 Inch Storm
- 33 % Void Ratio Stone
- 96 Tank Diameter (in.)
- 90 Tank Depth (in.)
- 4 Tank wall Thickness (in.)
- 100 Excavation per Tank (sq. ft.)
- 1 Depth of Stone Under Tank (ft.)

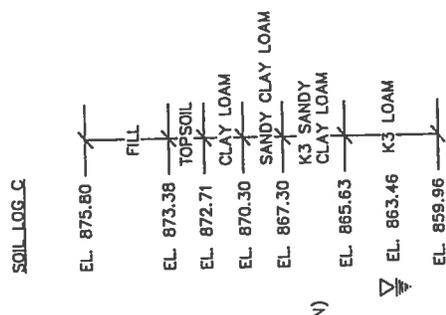
Tank Volume	=	3.14/4	x	Tank Diameter (ft)	x	Tank Diameter (ft)	x	Tank depth (ft)	=	316.617	Cu. Ft.
	=	0.785	x	7.333333	x	7.333333	x	7.500	=		
Stone Volume	=	(Excavation Size - Tank Area)	x	Tank Depth (ft)	x	Void Ratio			=	123.156	Cu. Ft.
	=	100.000 - 50.240	x	7.500	x	0.330			=		
Stone Base	=	Excavation Size (sq ft)	x	Depth of Stone (ft)	x	Void Ratio			=	33.000	Cu. Ft.
	=	100.000	x	1.000	x	0.330			=		
									+		
									=	472.773	Cu. Ft. per Drywell
Drywell Units Needed	=	Storage Necessary (Cu. Ft.)	/	Total Storage (Cu. Ft.)					=	8.63	
	=	4080.000	/	472.773					=	9	Drywell(s) Necessary

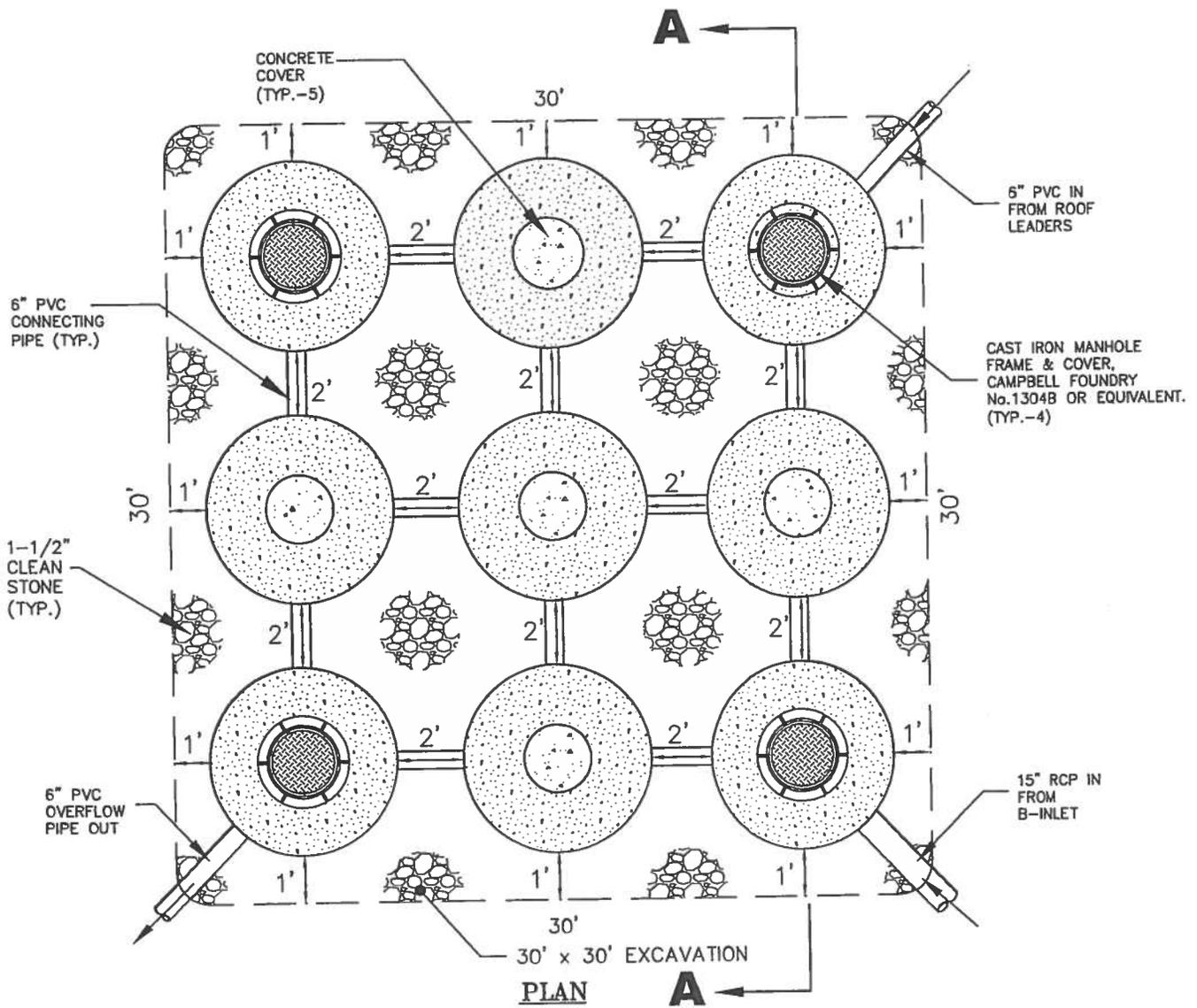
SOIL LOG C

EL. 875.80	FILL
EL. 873.38	TOPSOIL
EL. 872.71	CLAY LOAM
EL. 870.30	SANDY CLAY LOAM
EL. 867.30	K3 SANDY CLAY LOAM
EL. 865.63	K3 LOAM
EL. 863.46	K3
EL. 859.96	

SOIL LOG D

EL. 874.20	TOPSOIL
EL. 873.36	CLAY LOAM
EL. 869.61	SANDY CLAY LOAM
EL. 866.53	LOAM
EL. 862.53	
EL. 861.70	





DRYWELL DETAIL
(N.T.S.)

For Stormwater Management System

SOIL LOG C- Performed June 22, 2012

0" - 29" Fill
29" - 37" Original Topsoil 10YR 3/3
37" - 66" Clay Loam 5YR 5/8
5% Gravel, 5% Cobble, 2% Stone
Subangular Blocky, Friable
66" - 102" Sandy Clay Loam 7.5YR 5/8
5% Gravel, 5% Cobble, 2% Stone
Subangular Blocky, Friable
102" - 122" Sandy Clay Loam 7.5YR 5/6
5% Gravel, 10% Cobble, 5% Stone
Subangular Blocky, Friable
122" - 190" Loam 10YR 5/6
5% Gravel, 10% Cobble, 2% Stone
Single Grain, Loose

Sample @ 108, 128"

Roots to 133"

Mottling 148" - Bottom, Common Medium Distinct 2.5Y 7/2 to 7.5YR 5/8

Seepage @ 163"

No Ledge

SOIL LOG D- Performed June 22, 2012

0" - 10" Topsoil 10YR 3/3
10" - 55" Clay Loam 5YR 5/8
5% Gravel, 2% Cobble
Subangular Blocky, Friable
55" - 92" Sandy Clay Loam 7.5YR 5/8
5% Gravel, 5% Cobble, 5% Stone
Subangular Blocky, Friable
92" - 150" Loam 10YR 5/6
5% Gravel, 5% Cobble, 2% Stone
Single Grain Friable

Percolation Test PT1 @ 100" yields 10 min/in.

Roots to 133"

Mottling 140" - Bottom, Common Medium Distinct 2.5Y 7/2 to 7.5YR 5/8

Water @ 148" after 1 Hr.

No Ledge

COUNTY/MUNICIPALITY

Morris/ Chester Borough

Block 103
Lot 51

APPLICATION FOR PERMIT TO CONSTRUCT/ALTER/REPAIR
AN INDIVIDUAL SUBSURFACE SEWAGE DISPOSAL SYSTEM

Form 3c. Soil Permeability Class Rating Data

1. Test Number	6	Replicate Letter	A
2. Sample Depth	108"	Soil Pit/ Boring Number	C
		Date Collected:	06/22/12
3. Coarse Fragment Content:			
Total Weight of Sample, W.T. grams			450.00
Weight of Material Retained on 2mm Sieve, W.C.F., Grams			122
Weight % Coarse Fragment (W.C.F./W.T. x 100):			27.1%
4. Oven Dried Weight (24 hours at 105 deg C) of			
40 Gram Air Dried Sample			39.50
5. Hydrometer Calibration, Rc			5.0 grams
6. Hydrometer Reading-40 Seconds, R1			17.0 grams
Temperature of Suspension		68.0 deg F	
7. Corrected Hydrometer Reading, R1'			12.0 grams
8. Hydrometer Reading-2 Hours, R2			14.5 grams
Temperature of Suspension		68.0 deg F	
9. Corrected Hydrometer Reading, R2'			9.5 grams
10. % Sand = (Wt. - R1') / Wt. x 100			
= (39.5 - 12.0) / 39.5 x 100 =			69.6%
11. % Clay = R2' / Wt. x 100 = (
= (9.5 / 39.5) x 100 =			24.1%
12. Sieve Analysis:			
a: Oven Dry Weight (2 hrs, 105 deg C) Total Sand Fraction			
(Soil Retained in 0.047 mm sieve)			26.7 grams
b: Weight of Fine Plus Very Fine Sand Fraction			
(Sand Passing 0.25 mm Sieve)			8.8 grams
c: % Fine Plus Very Fine Sand (b/a)			33.0%
13. Soil Morphology (Natural Soil Samples Only)			

Structure of Soil Samples Tested: Subangular Blocky

Consistence of Soil Samples Tested: Dry: Friable
Moist: Friable

14. Soil Permeability Class Rating (Based upon average textural analysis of this replicated and other replicate samples) K3

15: I hereby certify that the information furnished on form 3c of this application is true and accurate. I am aware that falsification of data is a violation of Water Pollution Control Act (N.J.S.A. 58:10A-1 et seq.) and is subject to penalties as prescribed in N.J.A.C. 7:14-8.

Signature of Soil Evaluator

Date: 7/6/12

Signature of Professional Engineer

James Glasson, NJPE 37703

For: Civil Engineering, Inc., 1 Cove Street Budd Lake, NJ 07828

COUNTY/MUNICIPALITY

Morris/ Chester Borough

Block 103
Lot 51

APPLICATION FOR PERMIT TO CONSTRUCT/ALTER/REPAIR
AN INDIVIDUAL SUBSURFACE SEWAGE DISPOSAL SYSTEM

Form 3c. Soil Permeability Class Rating Data

1. Test Number	6	Replicate Letter	B
2. Sample Depth	108"	Soil Pit/ Boring Number	C
		Date Collected:	06/22/12
3. Coarse Fragment Content:			450.00
Total Weight of Sample, W.T. grams			122
Weight of Material Retained on 2mm Sieve, W.C.F., Grams			27.1%
Weight % Coarse Fragment (W.C.F./W.T. x 100):			
4. Oven Dried Weight (24 hours at 105 deg C) of			39.50
40 Gram Air Dried Sample			
5. Hydrometer Calibration, Rc			5.0 grams
6. Hydrometer Reading-40 Seconds, R1			17.1 grams
Temperature of Suspension		68.0 deg F	
7. Corrected Hydrometer Reading, R1'			12.1 grams
8. Hydrometer Reading-2 Hours, R2			14.3 grams
Temperature of Suspension		68.0 deg F	
9. Corrected Hydrometer Reading, R2'			9.3 grams
10. % Sand = (Wt. - R1') / Wt. x 100			
= (39.5 - 12.1) / 39.5 x 100 =		39.5 x 100 =	69.4%
11. % Clay = R2' / Wt. x 100 = (9.3 / 39.5 x 100 =	23.5%
12. Sieve Analysis:			
a: Oven Dry Weight (2 hrs, 105 deg C) Total Sand Fraction			26.9 grams
(Soil Retained in 0.047 mm sieve)			
b: Weight of Fine Plus Very Fine Sand Fraction			8.6 grams
(Sand Passing 0.25 mm Sieve)			32.0%
c: % Fine Plus Very Fine Sand (b/a)			

13. Soil Morphology (Natural Soil Samples Only)

Structure of Soil Samples Tested:

Subangular Blocky

Consistence of Soil Samples Tested:

Dry: Friable
Moist: Friable

14. Soil Permeability Class Rating (Based upon average textural analysis of this replicated and other replicate samples)

K3

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Signature of Soil Evaluator

Date: 7/6/12

Signature of Professional Engineer

James Glassch, NJPE 37703

For: Civil Engineering, Inc., 1 Cove Street Budd Lake, NJ 07828

COUNTY/MUNICIPALITY Morris/ Chester Borough Block 103
 Lot 51

APPLICATION FOR PERMIT TO CONSTRUCT/ALTER/REPAIR
 AN INDIVIDUAL SUBSURFACE SEWAGE DISPOSAL SYSTEM

Form 3c. Soil Permeability Class Rating Data

1. Test Number 7 Replicate Letter A
 2. Sample Depth 128" Soil Pit/ Boring Number C
 Date Collected: 06/22/12

3. Coarse Fragment Content:
 Total Weight of Sample, W.T. grams 450.00
 Weight of Material Retained on 2mm Sieve, W.C.F., Grams 106
 Weight % Coarse Fragment (W.C.F./W.T. x 100): 23.6%

4. Oven Dried Weight (24 hours at 105 deg C) of
 40 Gram Air Dried Sample 39.70

5. Hydrometer Calibration, Rc 5.0 grams
 6. Hydrometer Reading-40 Seconds, R1 23.0 grams
 Temperature of Suspension 68.0 deg F
 7. Corrected Hydrometer Reading, R1' 18.0 grams
 8. Hydrometer Reading-2 Hours, R2 10.0 grams
 Temperature of Suspension 68.0 deg F
 9. Corrected Hydrometer Reading, R2' 5.0 grams

10. % Sand = (Wt. - R1') / Wt. x 100
 = (39.7 - 18.0) / 39.7 x 100 = 54.7%

11. % Clay = R2' / Wt. x 100 = (5.0 / 39.7) x 100 = 12.6%

12. Sieve Analysis:
 a: Oven Dry Weight (2 hrs, 105 deg C) Total Sand Fraction
 (Soil Retained in 0.047 mm sieve) 25.7 grams
 b: Weight of Fine Plus Very Fine Sand Fraction
 (Sand Passing 0.25 mm Sieve) 8.8 grams
 c: % Fine Plus Very Fine Sand (b/a) 34.2%

13. Soil Morphology (Natural Soil Samples Only)

Structure of Soil Samples Tested: Single Grain

Consistence of Soil Samples Tested: Dry:
 Moist: Loose

14. Soil Permeability Class Rating (Based upon average textural
 analysis of this replicated and other replicate samples) K3

15: I hereby certify that the information furnished on form 3c of this application is true and accurate.
 I am aware that falsification of data is a violation of Water Pollution Control Act (N.J.S.A.
 58:10A-1 et seq.) and is subject to penalties as prescribed in N.J.A.C. 7:14-8.

Signature of Soil Evaluator

Date: 7/6/12

Signature of Professional Engineer

James Glasson, NJPE 37703

For: Civil Engineering, Inc., 1 Cove Street Budd Lake, NJ 07828

COUNTY/MUNICIPALITY

Morris/ Chester Borough

Block
Lot

103
51

APPLICATION FOR PERMIT TO CONSTRUCT/ALTER/REPAIR
AN INDIVIDUAL SUBSURFACE SEWAGE DISPOSAL SYSTEM

Form 3c. Soil Permeability Class Rating Data

1. Test Number 7 Replicate Letter B
 2. Sample Depth 128" Soil Pit/ Boring Number C
 Date Collected: 06/22/12

3. Coarse Fragment Content: 450.00
 Total Weight of Sample, W.T. grams
 Weight of Material Retained on 2mm Sieve, W.C.F., Grams 106
 Weight % Coarse Fragment (W.C.F./W.T. x 100): 23.6%

4. Oven Dried Weight (24 hours at 105 deg C) of 39.70
 40 Gram Air Dried Sample

5. Hydrometer Calibration, Rc 5.0 grams
 6. Hydrometer Reading-40 Seconds, R1 23.2 grams
 Temperature of Suspension 68.0 deg F
 7. Corrected Hydrometer Reading, R1' 18.2 grams
 8. Hydrometer Reading-2 Hours, R2 10.1 grams
 Temperature of Suspension 68.0 deg F
 9. Corrected Hydrometer Reading, R2' 5.1 grams

10. % Sand = (Wt. - R1') / Wt. x 100
 = (39.7 - 18.2) / 39.7 x 100 = 54.2%

11. % Clay = R2' / Wt. x 100 = (5.1 / 39.7) x 100 = 12.8%

12. Sieve Analysis:
 a: Oven Dry Weight (2 hrs, 105 deg C) Total Sand Fraction 25.5 grams
 (Soil Retained in 0.047 mm sieve)
 b: Weight of Fine Plus Very Fine Sand Fraction 8.6 grams
 (Sand Passing 0.25 mm Sieve)
 c: % Fine Plus Very Fine Sand (b/a) 33.7%

13. Soil Morphology (Natural Soil Samples Only)

Structure of Soil Samples Tested: Single Grain

Consistence of Soil Samples Tested: Dry: Loose
Moist: Loose

14. Soil Permeability Class Rating (Based upon average textural analysis of this replicated and other replicate samples) K3

15: I hereby certify that the information furnished on form 3c of this application is true and accurate. I am aware that falsification of data is a violation of Water Pollution Control Act (N.J.S.A. 58:10A-1 et seq.) and is subject to penalties as prescribed in N.J.A.C. 7:14-8.

Signature of Soil Evaluator

Date: 7/6/12

Signature of Professional Engineer

James Glaston, NJPE 37703

For: Civil Engineering, Inc., 1 Cove Street Budd Lake, NJ 07828

THIS IS A PRE-DRAFT COURTESY COPY OF THIS RULE TO BE EFFECTIVE ON APRIL 2, 2012.
 ALL OF THE DEPARTMENT'S RULES ARE COMPILED IN TITLE 7 OF THE NEW JERSEY
 ADMINISTRATIVE CODE.

- a. Oven Dry Wt. (2 hrs., 105°C) Total Sand Fraction (Soil Retained in 0.047 mm Sieve), grams _____
- b. Wt. of Fine Plus Very Fine Sand Fraction (Sand Passing 0.25 mm Sieve), grams _____
- c. % Fine Plus Very Fine Sand (b/a) _____
- 14. Soil Morphology (Natural Soil Samples Only):
 Structure of Soil Horizon Tested _____
 Consistence of Soil Horizon Tested: Dry ___ Moist ___
- 15. Soil Permeability Class Rating (Based upon average textural analysis of this replicate and other replicate samples) _____
- 16. I hereby certify that the information furnished on Form 3c of this application is true and accurate. I am aware that falsification of data is a violation of the Water Pollution Control Act (N.J.S.A. 58:10A-1 et seq) and is subject to penalties as prescribed in N.J.A.C. 7:14-8.
 Signature of Site Evaluator _____ Date _____
 Signature of Professional Engineer _____ License # _____

Form 3d. Percolation Test Data

- 1. Test Number PT1 Replicate (Letter) ___ Date Tested 6/22/12
- 2. Depth 100 "
- 3. Pre-soak: _____
 Sandy Textured Soil Only, Shortened Pre-soak—Indicate Time Required for 12 Inches of Water to Drain After Second Filling, Minutes 40
 Four Hour Pre-soak Completed—Indicate Result:
 ___ Test Hole Drained Within 16 to 24 Hours After Pre-soak
 ___ Test Hole Did Not Drain Within 24 Hours After Pre-soak
- 4. Rate of Fall Data:
 a. Time Interval Selected, Minutes 10
 b. Record the Drop in Water Level During Each Time Interval to the Nearest 1/10th-Inch On the Lines Below:

Depth of Water, Start of Interval (inches)	Depth of Water, End of Interval (inches)	Drop in Water Level (Inches)
9	7 3/4	1 1/4 "
7 3/4	6 3/4	1 "
6 3/4	5 3/4	1 "
4 3/4	3 3/4	1 "
3 3/4	2 3/4	1 "
2 3/4	1 3/4	1 "

- 5. Percolation Rate:
 a. Time, minutes, Required for a Six-inch Drop in Water Level 60
 b. Percolation Rate = $a/6 = 60/6 = 10$ min/in
- 6. I hereby certify that the information furnished on Form 3d of this application is true and accurate. I am aware that falsification of data is a violation of the Water Pollution Control Act (N.J.S.A. 58:10A-1 et seq.) and is subject to penalties as prescribed in N.J.A.C. 7:14-8.
 Signature of Site Evaluator _____ Date 7/6/12
 Signature of Professional Engineer _____ License # 37703

STORM SEWER STRUCTURE WORKSHEET

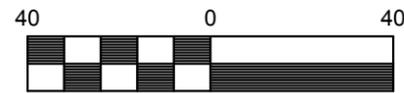
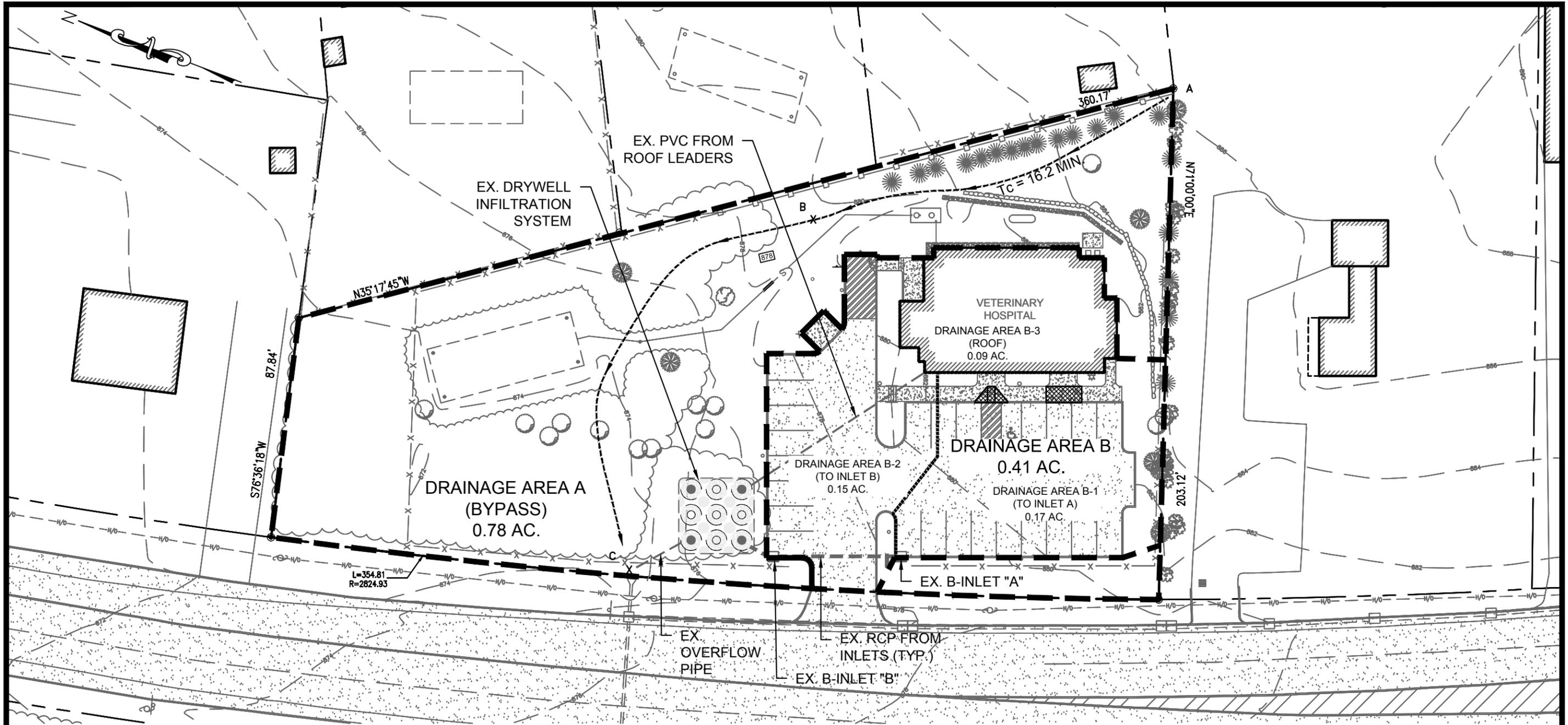
STRUCTURE	AREA#	AREA (ac)	"C" COEFFICIENT	TIME OF CONCENTRATION												
B INLET	B-1	.17 ac	<table style="margin-left: auto; margin-right: auto; border: none;"> <tr> <td></td> <td style="text-align: center;"><u>Area</u></td> <td style="text-align: center;"><u>C</u></td> </tr> <tr> <td>PAV'T</td> <td style="text-align: center;">.13</td> <td style="text-align: center;">.99</td> </tr> <tr> <td>GLASS</td> <td style="text-align: center;">.04</td> <td style="text-align: center;">.45</td> </tr> <tr> <td colspan="3" style="text-align: center; padding-top: 10px;">$C = .86$</td> </tr> </table>		<u>Area</u>	<u>C</u>	PAV'T	.13	.99	GLASS	.04	.45	$C = .86$			10 MIN. MINIMUM
	<u>Area</u>	<u>C</u>														
PAV'T	.13	.99														
GLASS	.04	.45														
$C = .86$																
B INLET	B-2	.15 ac	<table style="margin-left: auto; margin-right: auto; border: none;"> <tr> <td></td> <td style="text-align: center;"><u>Area</u></td> <td style="text-align: center;"><u>C</u></td> </tr> <tr> <td>PAV'T</td> <td style="text-align: center;">.13</td> <td style="text-align: center;">.99</td> </tr> <tr> <td>GLASS</td> <td style="text-align: center;">.02</td> <td style="text-align: center;">.45</td> </tr> <tr> <td colspan="3" style="text-align: center; padding-top: 10px;">$C = .92$</td> </tr> </table>		<u>Area</u>	<u>C</u>	PAV'T	.13	.99	GLASS	.02	.45	$C = .92$			10 MIN. MINIMUM
	<u>Area</u>	<u>C</u>														
PAV'T	.13	.99														
GLASS	.02	.45														
$C = .92$																
Roof	B-3	.09	.99	10 MIN. MINIMUM												

STORM SEWER TABULATION

PROJECT NO: 4923 PROJECT NAME: BLACK RIVER
 LOCATION: RT 206 CHESTER BOROUGH NJ
 COMPUTED BY: AG DATE: _____
 CHECKED BY: _____ DATE: _____
 DESIGN STORM FREQUENCY: 25 YEAR _____

CIVIL ENGINEERING, INC.
 1 COVE STREET
 BUDD LAKE, N.J. 07828
 Telephone: (973) 426-1776
 Fax: (973) 426-0716

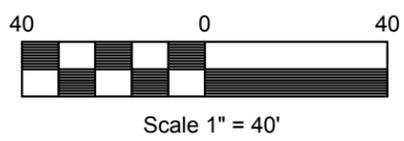
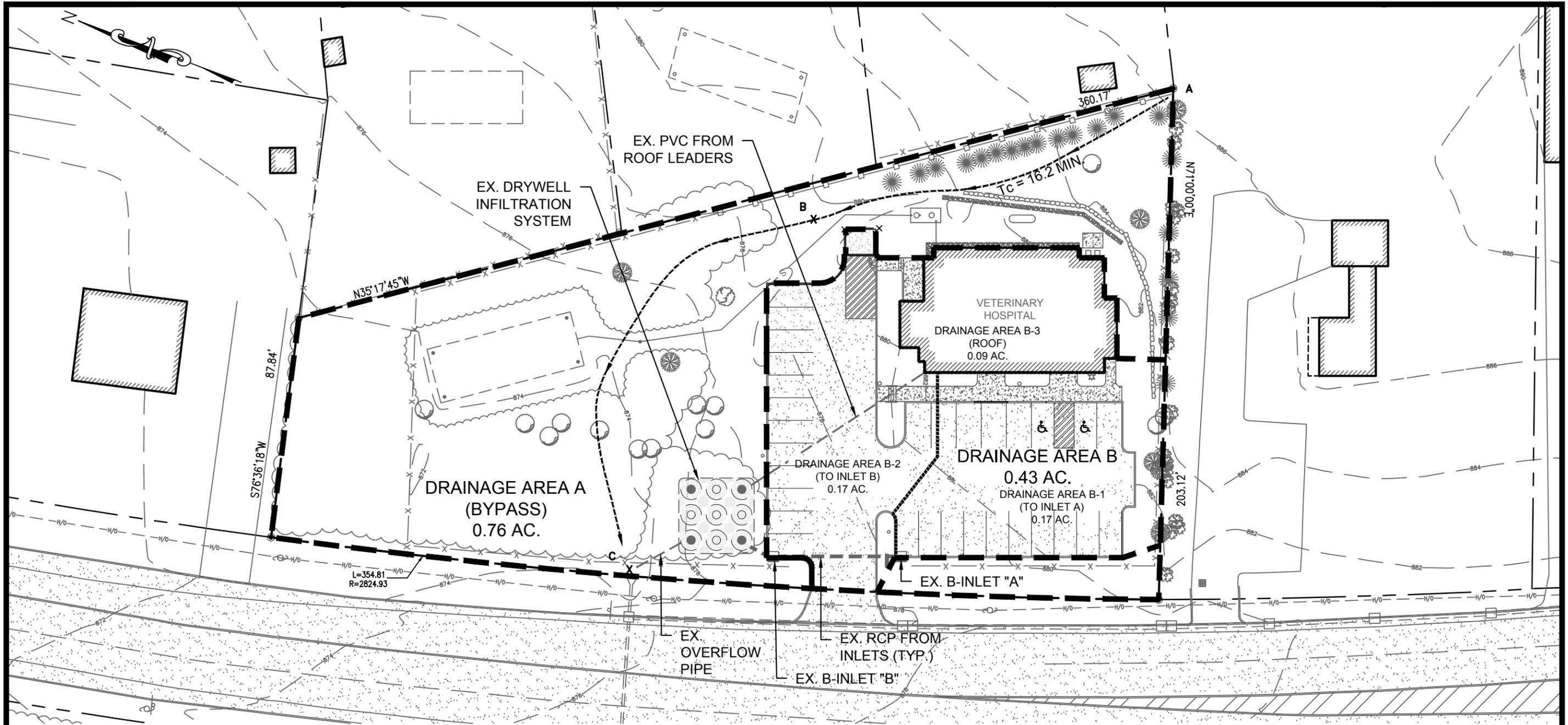
LOCATION		RUNOFF DATA										SEWER DESIGN DATA								
UPSTREAM	DOWNSTREAM	STRUCTURE NUMBER	AREA (AC)	WEIGHTED RAINFALL COEFFICIENT	INCREMENTAL AREA	TOTAL T.C. OR LONGEST THROUGH PIPE T.C.				TIME OF CONCENTRATION		PEAK RAINFALL INTENSITY I _P (INCHES)	DIAMETER (FT)	LENGTH (%)	SLOPE (CFS)	CAPACITY AT FULL FLOW V ₄ (PPS)	VELOCITY AT FULL FLOW V ₄ (FPS)	MANNINGS 'n'	PIPE MATERIAL ²	MANNINGS 'n'
						ORIGINATING AREA T.C. (MIN)	(8)	(MIN)	(9)	(MIN)	(10)									
(1)	(2)	(3)	(4)	(5) = (3)x(4)	(6) = Σ(5)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	INVERT	INVERT
B inset "A"	B inset "B"	.17	.86	.146	10	-	10	5.8	.85	15	47	4.2	13.5	11.0	3.3	875.13	873.2	.24		
B inset "B"	INJECT	.15	.92	.138	10	.25	10.25	5.75	1.63	15	8	1.0	6.5	5.3	3.7	873.10	873.02	.03		
Ref	INJECT	.09	.99	.089	10	-	10	5.8	.51	8"	87	3.4	3.0	8.0	4.4	876	873.02			




 140 WEST MAIN STREET HIGH BRIDGE, NJ 08829
 (908) 238-0544 FAX: (908)238-9572
 A PROFESSIONAL ASSOCIATION

TITLE:
EXISTING DRAINAGE AREA PLAN

LOCATION: 114 US HIGHWAY 206 NORTH BLOCK 103 LOT 51 BOROUGH OF CHESTER MORRIS COUNTY, NJ	SCALE: 1"=40'	PROJECT NO.: 19003
	DATE: 5/1/19	DRAWN BY: EM
	FILENAME: EX.DRAINAGE	SHEET NO.: 1 OF 2



		140 WEST MAIN STREET HIGH BRIDGE, NJ 08829	
		(908) 238-0544 FAX: (908)238-9572	
A PROFESSIONAL ASSOCIATION			
TITLE:			
POST-DEVELOPMENT DRAINAGE AREA PLAN			
LOCATION:		SCALE:	PROJECT NO.:
114 US HIGHWAY 206 NORTH		1"=40'	19003
BLOCK 103 LOT 51		DATE:	DRAWN BY:
BOROUGH OF CHESTER		5/1/19	EM
MORRIS COUNTY, NJ		FILENAME:	SHEET NO.:
		PROP.DRAINAGE	2 OF 2



CIVIL ENGINEERING
ENVIRONMENTAL
SURVEYING
LANDSCAPE ARCHITECTURE
GEOTECHNICAL

STORMWATER MANAGEMENT REPORT

Black River Veterinary Hospital
114 US Highway 206 North
Chester Borough
Block 103, Lot 51
Morris County, New Jersey

Prepared For:
Tack Veterinary Holdings, LLC
C/O Douglas Tack

May 1, 2019



John Hansen
Professional Engineer
N.J. P.E. No. 24GEO4194500



Headquarters
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Clinton | Asbury Park | Denville | Philadelphia

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LIST OF APPENDICIES

Appendix A: Civil Engineering Inc. Stormwater Management Report 11/30/12
(Attached Separately)

Appendix B: Runoff Coefficient Worksheet

Appendix C: Existing and Proposed Hydrologic Analysis and Runoff Quantity
Calculations

Appendix D: Existing and Proposed Drywell Storage Volume Requirements

Appendix E: Drainage Area Plans (Attached Separately)



1. INTRODUCTION

Engineering & Land Planning Associates, Inc. has prepared this Stormwater Management Report on behalf of Tack Veterinary Holdings, LLC, to document the design methodology and associated calculations for the project's stormwater management for the proposed site improvements to be constructed in Chester Borough, Morris County, New Jersey. The subject property is located along the northbound side of Route 206 and is known as Lot 51, Block 103 as shown on Sheet 15 of the borough of Chester Tax Maps.

An existing drywell infiltration system currently stores and infiltrates the stormwater runoff from the developed portion of the site. This system was designed for the site improvements that were proposed in a previous site plan application as shown on a plan set entitled "Preliminary and Final Site Plans for Lot 51 Block 103 'Black River Veterinary Hospital' Situated In: The Borough of Chester, Morris County, New Jersey", prepared by Civil Engineering, Inc., dated 7/6/12 and last revised 12/30/12.

The proposed limit of disturbance is approximately 3,000 square feet (0.068 acres) with a net impervious coverage increase of 832 square feet (0.019 acres) in connection with this application. By definition, the project is not a "major development" as defined by N.J.A.C. 7:8. The stormwater management analysis for this project utilizes the same methodology that was proposed in the stormwater report for the previous site plan application for the existing drywell system. The slight increase in impervious coverage will affect the peak flows for the 2, 10, and 100 year storms. The purpose of this report is to present an analysis of the existing stormwater management system to determine if the existing drywell system is sized large enough to accommodate the increase in peak flows or if additional stormwater management is required.



2. PROJECT DESCRIPTION

2.1 Existing Conditions

The site, identified as Block 103, Lot 51, is located in Chester Borough, Morris County, New Jersey along the northbound side of Route 206 between the intersections with Route 24 and Melville Place. The property consists of approximately 1.19 acres and is developed with a Veterinary Hospital and associated parking lot and utilities. The rest of the lot is wooded with some maintained lawn area. It is trapezoidal in shape and bordered by residential properties.

As per the design plans by Civil Engineering, Inc., the existing stormwater drywell system contains nine (9), 8 foot diameter drywell units that are 7.5 feet in depth and encased in a 30 by 30 square foot stone bed extending 1 foot below the bottom of the drywells. The existing drywell system has a storage volume of 4,254.957 cubic feet. Based on the previous stormwater report entitled "Stormwater Management Report for 'Black River Veterinary Hospital' Lot 51 Block 103 Borough of Chester Morris County, New Jersey", prepared by Civil Engineering Inc., dated July 06, 2012 and last revised November 30, 2012, the drywell system is currently designed to store and infiltrate all of the water that is collected from its drainage area from the 100 year storm event. The drywell system collects runoff from the developed portion of the site through concrete pipe conveying water from two inlets in the parking lot and a PVC pipe from the roof leaders of the veterinary hospital building.

The stormwater runoff that is generated from the undeveloped portions of the site flows undetained in a northwesterly direction across the site and discharges to an existing 18 inch pipe within a low point on the lot which drains to the NJDOT Route 206 Drainage System. Overflow from the drywell system is directed to the same low point via a PVC pipe. Refer to Appendix A for the previous stormwater report.

2.2 Proposed Conditions

The proposed project will include an expansion of the existing asphalt parking lot and relocation of the trash enclosure and associated curbing, fencing, and lighting. The project also proposes restriping of the parking stalls to increase the number of stalls and rearrange the location of the ADA stalls to an area that is more appropriate for handicap access to the veterinary hospital.

The existing stormwater drywell infiltration system shall be used to store and infiltrate the runoff from the proposed improvements.

2.3 Soil Conditions

Per the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) Web Soil Survey, the site is contained entirely in the Gladstone Gravelly Loam, 3-8% slopes (GkaoB) soils which are in hydrological soil group B.



Soil profile pits were recorded by Civil Engineering, Inc. on 06/22/12, and soil permeability class ratings as well as a percolation test were performed to determine the permeability rate. The soil logs report primarily clay-sandy clay loam to approximately 10 feet below grade, underlain by Loam. The soil permeability results from around 8.5 to 10.5 feet below grade reported K3 soils. Coarse fragments throughout the soil profile range from 12-20%. Some seepage and mottling was encountered starting at 140 inches. Soil logs and Soil Permeability Class Ratings are included in Appendix A as part of the prior stormwater management report.

3. METHODOLOGY

3.1 Stormwater Runoff Calculation Methodology

The stormwater quantity runoff analysis has been performed in accordance with the previous stormwater analysis performed by Civil Engineering, Inc. The previous analysis utilized the rational method to calculate peak flows while the required storage was based upon the modified rational method. The exact same methodology was followed in order to compare predevelopment and post development peak flows and to see if the existing stormwater drywell system is sized to store the 100 year storm volume of runoff generated from the revised drainage area.

The most hydraulically distant flow path was analyzed in the previous stormwater report by Civil Engineering Inc. (Appendix A) to determine the most appropriate time of concentrations. The proposed site improvements will have no effect on the time of concentrations that were calculated in the previous stormwater report. The most hydraulically distant flow path from one drainage area will not be disturbed, and the minimum allowed time of concentrations were used for the other drainage area due to the short flow path. New CN values were calculated for each drainage area. The summary of results and supporting calculations for the existing and proposed stormwater quantity runoff analysis can be found in Appendices B-C of this report.

4. STORMWATER ANALYSIS

4.1 Existing Conditions Stormwater Runoff Quantity

The Existing Drainage Area Plan (Appendix E) illustrates the existing drainage areas on site. The site has been analyzed as two distinct drainage areas. The existing drainage area plan is identical to the proposed post development drainage area plan from 2012 entitled "Post Development Drainage Area Plan for Lot 51 Block 103 Borough of Chester Morris County, New Jersey", prepared by Civil Engineering Inc., dated 7/6/12 and last revised 11/30/12.

Existing Drainage Area A is the area of the project site that bypasses the existing stormwater drywell system and drains directly to the 18 inch pipe that discharges to the NJDOT Route 206 Drainage System. It is approximately 0.78 acres in size and consists almost entirely of pervious cover.



Existing Drainage Area B is the developed portion of the site that is collected by inlets and roof drains and then conveyed to the drywell system. This area consists of the veterinary hospital and parking lot. Existing Drainage Area B is comprised of three sub drainage areas. Sub Drainage Area B-1 drains to a type-B inlet identified as “B-inlet A”, and Sub Drainage Area B-2 drains to a type-B inlet identified as “B-inlet B”. The type-B inlets are connected and sent to the drywell system by reinforced concrete pipe. Sub Drainage Area B-3 consists of the roof area which is captured and sent to the drywells by PVC pipe.

The runoff coefficients (C), time of concentrations (Tc), and peak flows for the 2, 10, and 100 year storms for the existing drainage areas have been calculated utilizing the rational method for each drainage area.

The pre-development runoff from the drainage areas is listed in the following table:

Drainage Area	2-year Storm	10-year Storm	100-year Storm
EXDA-A	1.00 CFS	1.36 CFS	1.89 CFS
EXDA-B	1.58 CFS	2.19 CFS	3.02 CFS

Refer to Appendices B-C for a summary of the composite runoff coefficients and pre-development peak discharge rates for the 2, 10, and 100 year storms.

4.2 Existing Conditions Drywell Storage

The existing drywell infiltration system was previously designed by Civil Engineering, Inc. to store and infiltrate the 100 year runoff generated from existing Drainage Area B. The storage required was calculated using the Modified Rational Method. Refer to page 19 in Appendix A for a summary of the storage requirements and page 23 for the drywell calculations. The existing drywell system has a total storage volume of 4,254.957 cubic feet. This was sized based on the volume of water generated from Drainage Area B for the 100 year storm with a duration of 45 minutes. However, there was an inconsistency in the previous report for the storage volume needed for this storm duration. The Civil Engineering, Inc. report calculated a required storage volume of 4,080 cubic feet, whereas our firm calculated the storage volume to be 3,672 cubic feet based on the methodology that was used. Refer to Appendix D for the required drywell storage volume calculations.

4.3 Proposed Conditions Stormwater Runoff Quantity

The Proposed Drainage Area Plan (Appendix E), illustrates the proposed drainage areas for the post-development condition. The proposed development includes a slight expansion of Drainage Area B and reduction in Drainage Area A. Refer to Appendix B for the post development runoff coefficients.

The most hydraulically distant flow path for Drainage Area A is unaffected by the proposed development and is, therefore, to remain the same as the prior Civil Engineering, Inc. stormwater report. The prior stormwater report used the minimum time of concentration for Drainage Area B due to the short flow paths to the inlets and added 0.25 minutes for the time to flow through the pipe from inlet to the drywells. The proposed development adds only minimal distance to the flow path to “B-inlet B”, so the time of concentration will remain the same.



The post development runoff from the drainage areas is listed in the following table:

Drainage Area	2-year Storm	10-year Storm	100-year Storm
PRDA-A	0.97 CFS	1.32 CFS	1.84 CFS
PRDA-B	1.68 CFS	2.32 CFS	3.20 CFS

Refer to Appendices B-C for a summary of the composite runoff coefficients and post development peak discharge rates for the 2, 10, and 100 year storms.

4.4 Proposed Conditions Drywell Storage

The same methodology used in the Civil Engineering, Inc. report was used to recalculate the required storage for the post development Drainage Area B. The post development storage required for the subject project is 3,888 cubic feet. Refer to Appendix D for the required drywell storage volume calculations. The existing drywell system has a total storage volume of 4,254.957 cubic feet.

5. CONCLUSIONS

In conclusion, the existing stormwater infiltration drywell system has sufficient storage volume to accommodate the additional stormwater runoff from the slight increase in drainage area and impervious coverage in connection with this project.



APPENDIX A
(Attached Separately)



APPENDIX B



Project: Black River Vet
 Location: 114 Route 206, Chester
Morris County, NJ

By: EM
 Date: 1-Apr-19
 Chk'd: JH
 Revised: _____

Watershed: Existing Drainage Area A

RUNOFF COEFFICIENT CALCULATIONS:
 (rational method)

Soil name and hydrologic group	Cover Description	C	Area (acres)	Product of CN x Area
GkaoB (B)	Woods	0.25	0.33	0.08
GkaoB (B)	Grass	0.45	0.44	0.20
GkaoB (B)	Impervious	0.99	0.01	0.01

'B' Soils:
 GkaoB

Totals =

0.78	0.29
------	------

Composite C = $\frac{0.29}{0.78}$ = 0.37

USE C = 0.37

Project: Black River Vet
 Location: 114 Route 206, Chester
Morris County, NJ

By: EM
 Date: 1-Apr-19
 Chk'd: JH
 Revised: _____

Watershed: Existing Drainage Area B

RUNOFF COEFFICIENT CALCULATIONS:
 (rational method)

Soil name and hydrologic group	Cover Description	C	Area (acres)	Product of CN x Area
GkaoB (B)	Grass	0.45	0.05	0.02
GkaoB (B)	Impervious	0.99	0.36	0.36

'B' Soils:
 GkaoB

Totals =

0.41	0.38
------	------

Composite C = $\frac{0.38}{0.41}$ = 0.92

USE C = 0.92

Project: Black River Vet
 Location: 114 Route 206, Chester
Morris County, NJ

By: EM
 Date: 1-Apr-19
 Chk'd: JH
 Revised: _____

Watershed: Proposed Drainage Area B

RUNOFF COEFFICIENT CALCULATIONS:
 (rational method)

Soil name and hydrologic group	Cover Description	C	Area (acres)	Product of CN x Area
GkaoB (B)	Grass	0.45	0.05	0.02
GkaoB (B)	Impervious	0.99	0.38	0.38

'B' Soils:
 GkaoB

Totals =

0.43	0.40
------	------

Composite C = $\frac{0.40}{0.43}$ = 0.93

USE C = 0.93

APPENDIX C



PEAK FLOWS SUMMARY SHEET

(Rational Method: $Q = C i A$)

EXISTING DRAINAGE AREA A

- I. Runoff Coefficient (C) = 0.37
- II. Time of Concentration (Tc) = 16.2 Minutes
(Taken from Civil Engineering, Inc. Stormwater Report, 11/30/12)
- III. Intensity (i): (Functions of Tc)
 1. 2 year storm: i = 3.46 in/hr
 2. 10 year storm: i = 4.71 in/hr
 3. 100 year storm: i = 6.56 in/hr
- IV. Area (A) = 0.78 Acres
- V. Peak Flows (Q):
 1. 2 year storm: $Q = 0.37 \times 3.46 \times 0.78 = 1.00$ CFS
 2. 10 year storm: $Q = 0.37 \times 4.71 \times 0.78 = 1.36$ CFS
 3. 100 year storm: $Q = 0.37 \times 6.56 \times 0.78 = 1.89$ CFS

EXISTING DRAINAGE AREA B

- I. Runoff Coefficient (C) = 0.92
- II. Time of Concentration (Tc) = 10.25 Minutes
(Taken from Civil Engineering, Inc. Stormwater Report, 11/30/12)
- III. Intensity (i): (Functions of Tc)
 1. 2 year storm: i = 4.20 in/hr
 2. 10 year storm: i = 5.80 in/hr
 3. 100 year storm: i = 8.00 in/hr
- IV. Area (A) = 0.41 Acres
- V. Peak Flows (Q):
 4. 2 year storm: $Q = 0.92 \times 4.20 \times 0.41 = 1.58$ CFS
 5. 10 year storm: $Q = 0.92 \times 5.80 \times 0.41 = 2.19$ CFS
 6. 100 year storm: $Q = 0.92 \times 8.00 \times 0.41 = 3.02$ CFS



PROPOSED DRAINAGE AREA A

- I. Runoff Coefficient (C) = 0.37
- II. Time of Concentration (Tc) = 16.2 Minutes
(Taken from Civil Engineering, Inc. Stormwater Report, 11/30/12)
- III. Intensity (i): (Functions of Tc)
 1. 2 year storm: i = 3.46 in/hr
 2. 10 year storm: i = 4.71 in/hr
 3. 100 year storm: i = 6.56 in/hr
- IV. Area (A) = 0.76 Acres
- V. Peak Flows (Q):
 1. 2 year storm: $Q = 0.37 \times 3.46 \times 0.76 = 0.97$ CFS
 2. 10 year storm: $Q = 0.37 \times 4.71 \times 0.76 = 1.32$ CFS
 3. 100 year storm: $Q = 0.37 \times 6.56 \times 0.76 = 1.84$ CFS

PROPOSED DRAINAGE AREA B

- I. Runoff Coefficient (C) = 0.93
- II. Time of Concentration (Tc) = 10.25 Minutes
(Taken from Civil Engineering, Inc. Stormwater Report, 11/30/12)
- III. Intensity (i): (Functions of Tc)
 7. 2 year storm: i = 4.20 in/hr
 8. 10 year storm: i = 5.80 in/hr
 9. 100 year storm: i = 8.00 in/hr
- IV. Area (A) = 0.43 Acres
- V. Peak Flows (Q):
 10. 2 year storm: $Q = 0.93 \times 4.20 \times 0.43 = 1.68$ CFS
 11. 10 year storm: $Q = 0.93 \times 5.80 \times 0.43 = 2.32$ CFS
 12. 100 year storm: $Q = 0.93 \times 8.00 \times 0.43 = 3.20$ CFS





APPENDIX D



DRYWELL STORAGE SUMMARY SHEET

(Modified Rational Method: $Q = C i A$)

EXISTING DRAINAGE AREA B

- I. Runoff Coefficient (C) = 0.92
- II. Time of Concentration (T_c) = 10.25 Minutes
(Taken from Civil Engineering, Inc. Stormwater Report, 11/30/12)
- III. IDF Curve 100 Year Intensity (i): (Functions of Storm Duration)
- a) 20 min. storm duration: i = 5.6 in/hr
 - b) 30 min. storm duration: i = 4.6 in/hr
 - c) 45 min. storm duration: i = 3.6 in/hr
 - d) 60 min. storm duration: i = 2.9 in/hr
 - e) 90 min. storm duration: i = 2.2 in/hr
 - f) 120 min. storm duration: i = 1.7 in/hr
 - g) 180 min. storm duration: i = 1.3 in/hr
 - h) 240 min. storm duration: i = 1.1 in/hr
- IV. Area (A) = 0.41 Acres
- V. Peak Flows (Q):
- a) 20 min. storm duration: $Q = 0.92 \times 5.6 \times 0.41 = 2.11$ CFS
 - b) 30 min. storm duration: $Q = 0.92 \times 4.6 \times 0.41 = 1.74$ CFS
 - c) 45 min. storm duration: $Q = 0.92 \times 3.6 \times 0.41 = 1.36$ CFS
 - d) 60 min. storm duration: $Q = 0.92 \times 2.9 \times 0.41 = 1.09$ CFS
 - e) 90 min. storm duration: $Q = 0.92 \times 2.2 \times 0.41 = 0.83$ CFS
 - f) 120 min. storm duration: $Q = 0.92 \times 1.7 \times 0.41 = 0.64$ CFS
 - g) 180 min. storm duration: $Q = 0.92 \times 1.3 \times 0.41 = 0.49$ CFS
 - h) 240 min. storm duration: $Q = 0.92 \times 1.1 \times 0.41 = 0.41$ CFS
- VI. Required Storage (S): (Matching Civil Engineering, Inc. Stormwater Report, 11/30/12 Methodology)
- a) 20 min. storm duration: $S = (20 \times 2.11 \times 0.5) \times 2 \times 60 = \underline{2,532}$ CF
 - b) 30 min. storm duration: $S = [(0.5 \times 20 \times 1.74) + (10 \times 1.74) + (0.5 \times 20 \times 1.74)] \times 60 = \underline{3,132}$ CF
 - c) 45 min. storm duration: $S = [(0.5 \times 20 \times 1.36) + (25 \times 1.36) + (0.5 \times 20 \times 1.36)] \times 60 = \underline{3,672}$ CF *
 - d) 60 min. storm duration: $S = [(0.5 \times 20 \times 1.09) + (40 \times 1.09) + (0.5 \times 20 \times 1.09)] \times 60 = \underline{3,924}$ CF

* Civil Engineering, Inc. report shows the 45 min. storm duration to generate 4,080 CF due to an inconsistency in the calculations. The drywell storage volume was determined by the 100 year, 45 min. storm duration. The required storage volume for the post development Drainage Area B shall be determined based on the same storm duration.



PROPOSED DRAINAGE AREA B

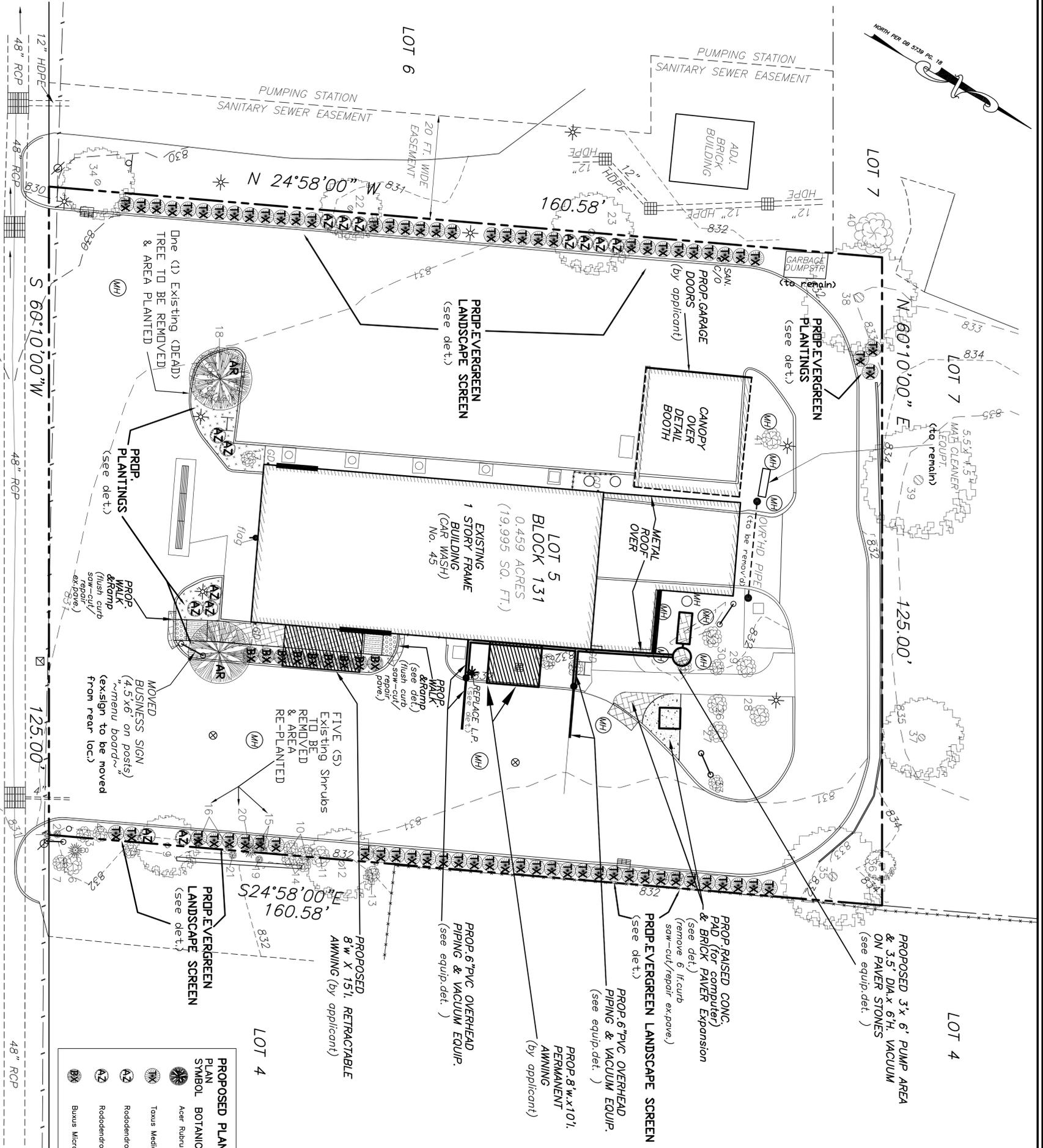
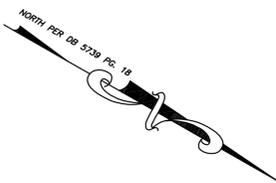
- I. Runoff Coefficient (C) = 0.93
- II. Time of Concentration (T_c) = 10.25 Minutes
(Taken from Civil Engineering, Inc. Stormwater Report, 11/30/12)
- III. IDF Curve 100 Year Intensity (i): (Functions of Storm Duration)
- a) 20 min. storm duration: i = 5.6 in/hr
 - b) 30 min. storm duration: i = 4.6 in/hr
 - c) 45 min. storm duration: i = 3.6 in/hr
 - d) 60 min. storm duration: i = 2.9 in/hr
 - e) 90 min. storm duration: i = 2.2 in/hr
 - f) 120 min. storm duration: i = 1.7 in/hr
 - g) 180 min. storm duration: i = 1.3 in/hr
 - h) 240 min. storm duration: i = 1.1 in/hr
- IV. Area (A) = 0.43 Acres
- V. Peak Flows (Q):
- i) 20 min. storm duration: $Q = 0.93 \times 5.6 \times 0.43 = 2.24$ CFS
 - j) 30 min. storm duration: $Q = 0.93 \times 4.6 \times 0.43 = 1.84$ CFS
 - k) 45 min. storm duration: $Q = 0.93 \times 3.6 \times 0.43 = 1.44$ CFS
 - l) 60 min. storm duration: $Q = 0.93 \times 2.9 \times 0.43 = 1.16$ CFS
 - m) 90 min. storm duration: $Q = 0.93 \times 2.2 \times 0.43 = 0.88$ CFS
 - n) 120 min. storm duration: $Q = 0.93 \times 1.7 \times 0.43 = 0.68$ CFS
 - o) 180 min. storm duration: $Q = 0.93 \times 1.3 \times 0.43 = 0.52$ CFS
 - p) 240 min. storm duration: $Q = 0.93 \times 1.1 \times 0.43 = 0.44$ CFS
- VI. Required Storage (S): (Matching Civil Engineering, Inc. Stormwater Report, 11/30/12 Methodology)
- a) 20 min. storm duration: $S = (20 \times 2.24 \times 0.5) \times 2 \times 60 = \underline{2,688}$ CF
 - b) 30 min. storm duration: $S = [(0.5 \times 20 \times 1.84) + (10 \times 1.84) + (0.5 \times 20 \times 1.84)] \times 60 = \underline{3,312}$ CF
 - c) 45 min. storm duration: $S = [(0.5 \times 20 \times 1.44) + (25 \times 1.44) + (0.5 \times 20 \times 1.44)] \times 60 = \underline{3,888}$ CF
 - d) 60 min. storm duration: $S = [(0.5 \times 20 \times 1.16) + (40 \times 1.16) + (0.5 \times 20 \times 1.16)] \times 60 = \underline{4,176}$ CF

The Post Develop Drainage Area B required storage volume is 3,888 CF.



APPENDIX E
(Attached Separately)





MAPLE AVENUE

PLAN VIEW



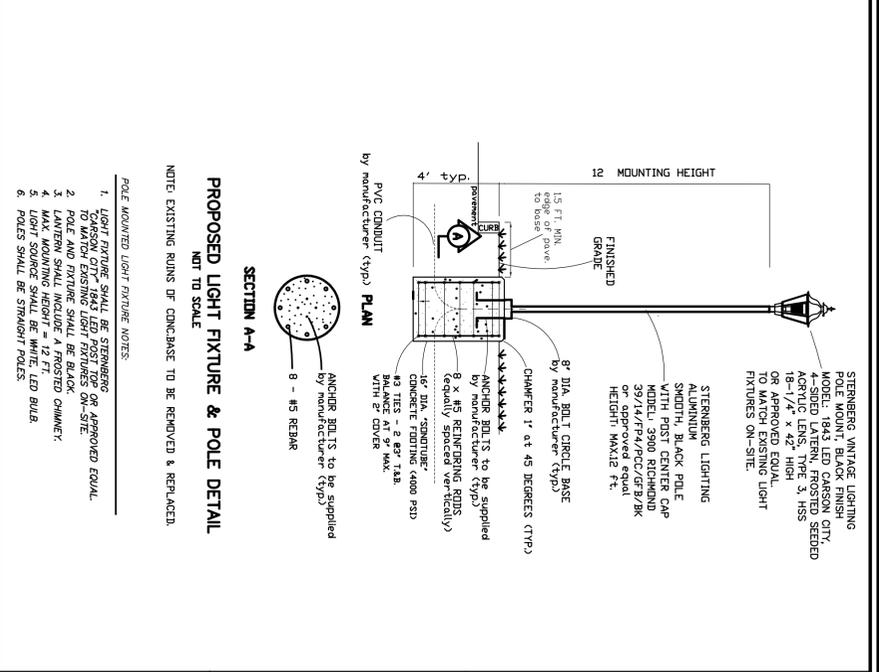
PROPOSED PLANTING SCHEDULE

PLAN SYMBOL	BOTANICAL NAME	COMMON NAME	QTY	SCREENING LOCATION	SIZE
	Acer Rubrum 'Red Sunset'	'Red Sunset' Maple	2	ON-SITE Landscape Area	2-2.5" CAL B&B
	Toxus Media Hickell	'Hicks' Yew	71	Westerly/Easterly Property Limits	24"-30", #7 CAN
	Rododendron 'Delaware Valley White'	'Delaware Valley White' Azalea	9	Northerly/Easterly Property Limits	18"-24", #7 CAN
	Rododendron 'Delaware Valley White'	'Delaware Valley White' Azalea	5	ON-SITE Landscape Area	18"-24", #7 CAN
	Buxus Microphylla 'Green Velvet'	'Green Velvet' Boxwood'	9	WALKWAY	24"-30"Height, B&B

EXISTING TREES & SHRUB SCHEDULE

No.	DESCRIPTION	QTY	REMARKS
1	1.5" DIA SHRUB (offsite)	23	16" MAPLE
2	1.5" DIA SHRUB	24	3" DIA SHRUB
3	3" DIA SHRUB	25	3" DIA SHRUB
4	3" DIA SHRUB (offsite)	26	4" DIA SHRUB
5	3" DIA SHRUB (offsite)	27	4" DIA SHRUB
6	3" DIA SHRUB (offsite)	28	4" DIA SHRUB
7	3" DIA SHRUB (offsite)	29	3" DIA SHRUB
8	3" SPRUCE (offsite)	30	3" DIA SHRUB
9	3" SPRUCE (offsite)	31	3" DIA SHRUB
10	3" SPRUCE (offsite)	32	4" DIA SHRUB
11	4" D. SHRUB	33	3" DIA SHRUB
12	14" MAPLE (offsite)	34	14" MAPLE (offsite)
13	3" DIA SHRUB (offsite)	35	20" OAK
14	2 x 3" DIA SHRUB (offsite)	36	16" LOCUST (offsite)
15	2 x 1.5" DIA SHRUB	37	16" LOCUST (offsite)
16	2 x 1.5" DIA SHRUB	38	22" OAK
17	10" MAPLE (OFF-AD)	39	8" DIA SHRUB (offsite)
18	10" MAPLE (OFF-AD)	40	8" DIA SHRUB (offsite)
19	1.5" DIA SHRUB (offsite)	41	4" DIA SHRUB
20	3" SPRUCE		
21	1.5" DIA SHRUB (offsite)		
22	12" MAPLE (offsite)		

denotes existing planting to be removed

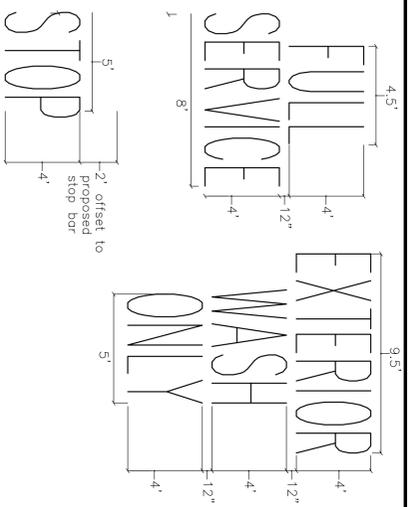


NOTE: SEE SHEET 6 FOR PLANTING DETAILS

SYMBOL LEGEND:

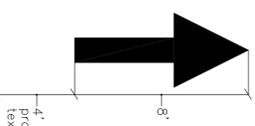
- denotes EXISTING LIGHT POLE (note: Fixture Mounting Height = 12 ft.)
- denotes PROPOSED POLE & FIXTURE
- denotes EXISTING USA FLAG
- denotes EXISTING MOVABLE SIGN

<p>YAMACCONE & ALDRICH, LLC</p> <p>Civil Engineers Land Surveyors & Professional Planners</p> <p>460 MAIN STREET, P.O. BOX 459 CHESTER, NEW JERSEY 07930 PHONE: 908-879-6646 FAX: 908-879-8591 M.L. STATE REG. NO. E.P.L.S. 00000000 OF AUTOMATIC NO. 3402934500</p>	<p>CANDICE J. DAVIS</p> <p>N.J. PROFESSIONAL ENGINEER NO. 246294322000 DATE 02/05/20</p>	<p>THE CAR WASH AT CHESTER, LLC</p> <p>AT SITE PLANS MINOR IMPROVEMENTS PROJECT</p> <p>LOT 5 ~ BLOCK 131 774 MAP SHEET No. 10 BOROUGH OF CHESTER MORRIS COUNTY, NEW JERSEY</p> <p>SHEET TITLE: PLANTING PLAN SHEET</p>	<p>NOT VALID WITHOUT SIGNATURE AND RAISED SEAL</p> <table border="1"> <tr><td>NO.</td><td>DATE</td><td>REVISION</td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </table>	NO.	DATE	REVISION										<p>DRAWN BY: <i>693CAD</i></p> <p>DATE: FEB. 05 2020</p> <p>CHECKED BY: <i>627</i></p> <p>SCALE: 1" = 10'</p> <p>F.B. 789/120</p> <p>W.D. 219/066</p> <p>FILE: 1/25/20/21906/20K/</p> <p>COMP FILE: 21906-SignPlan20</p> <p>SHEET 5 OF 6</p>
NO.	DATE	REVISION														



TEXT PAINTING DETAILS
NOT TO SCALE

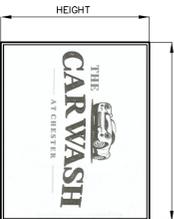
NOTE: ALL TEXT PAINT TO BE SOLID THERMOPLASTIC WHITE PAINT.
NOTE: SEE PLAN VIEW ON SHEET 4 FOR LOCATIONS.



ARROW DETAIL
NOT TO SCALE

NOTE: ALL ARROW PAINT TO BE SOLID THERMOPLASTIC WHITE PAINT.
NOTE: SEE PLAN VIEW ON SHEET 4 FOR LOCATIONS.

CUSTOM WALL-MOUNTED WOOD SITE SIGNS
NOT TO SCALE

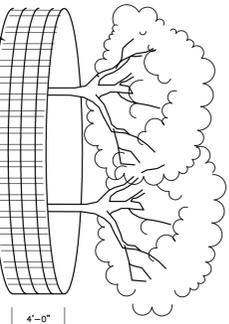


Colors:
White background
Blue and tan text
Blue art

NOTE: See business logo provided by the Applicant or approved equal.

NOTE: SEE SITE PLAN SHEET 4 FOR SIGN DIMENSIONS AND LOCATIONS.

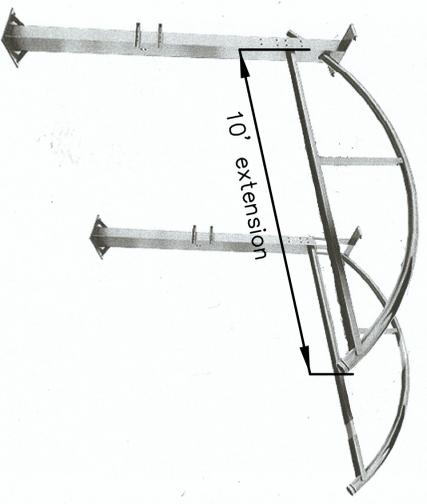
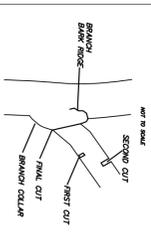
* FRONT MOUNTED SIGN: 10' W x 4' H
* MOUNT BOTTOM OF SIGN 1 FT. MIN. ABOVE GARAGE OPENING.
* SIDE MOUNTED SIGN: 8' W x 4' H
* MOUNT BOTTOM OF SIGN 5.0 FT. MIN. ABOVE GRADE



TREE PROTECTION DETAIL
NOT TO SCALE

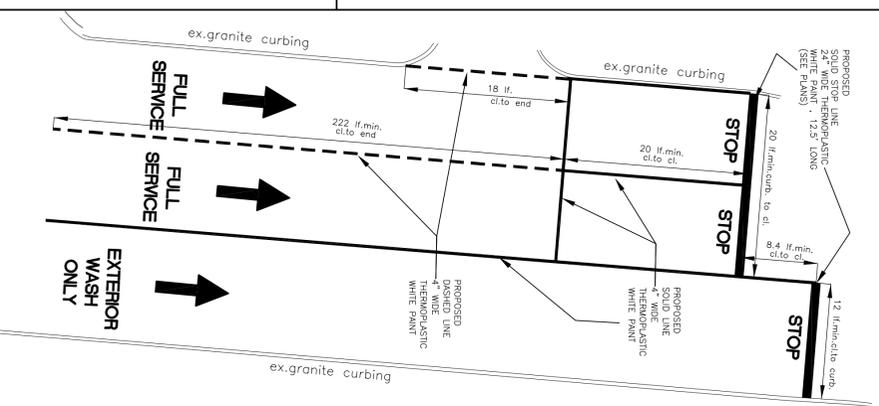
SNOW FENCE TO BE INSTALLED AT BASE LINE OF TREE CLUSTER TO BE PROTECTED.

REMOVAL OF TREE LIMB
NOT TO SCALE



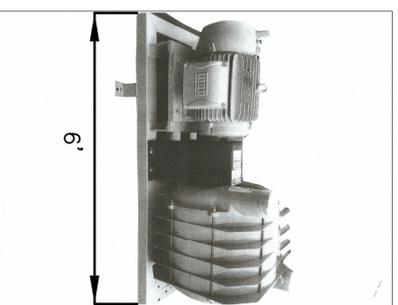
PROPOSED OVERHEAD EQUIPMENT DETAIL
NOT TO SCALE

NOTE: Provided by the Applicant.



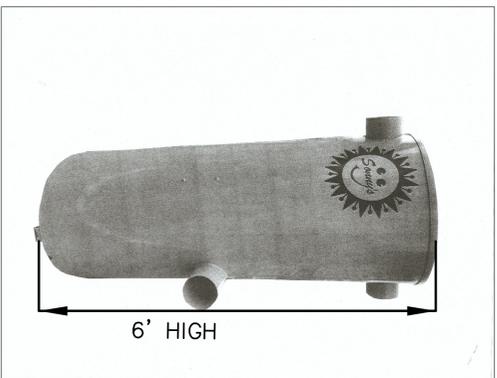
LANE PAINTING DETAILS
NOT TO SCALE

NOTE: ALL LANE STRIPING TO BE SOLID THERMOPLASTIC WHITE PAINT.
NOTE: SEE PLAN VIEW ON SHEET 4 FOR LOCATIONS.



PROPOSED 3' X 6' X 3' H. PUMP EQUIPMENT DETAIL
NOT TO SCALE

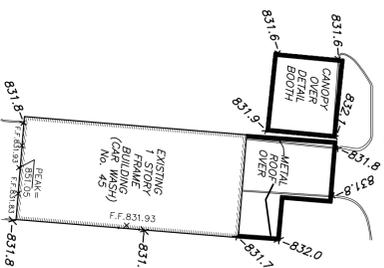
NOTE: Provided by the Applicant.



PROPOSED 3.5' DIA. X 6' H. EQUIPMENT DETAIL
NOT TO SCALE

NOTE: Provided by the Applicant.

AVERAGE GRADE & BUILDING HEIGHT CALCULATION DETAIL
NOT TO SCALE



BASED ON THE FIELD SURVEY, THE MEAN ELEVATION BETWEEN THE EAKES AND THE ROOF PEAK = (591.0' PEAK + 843.0' EAK)/2 = 842.0' FT. EL.

THE EXISTING AVERAGE GRADE AS MEASURED AT 7 POINTS ALONG THE FOUNDATION IS 831.8 FT. EL.

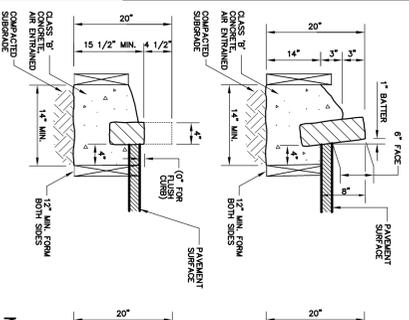
THEREFORE, THE EXISTING BUILDING HEIGHT EQUALS: 842.0 FT. EL. - 831.8 FT. EL. = 10.2 FT. < 35 FT. MAX. COMPRES

ACCESSORY BUILDING, DETAIL BOOTH

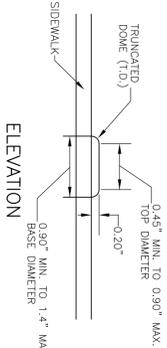
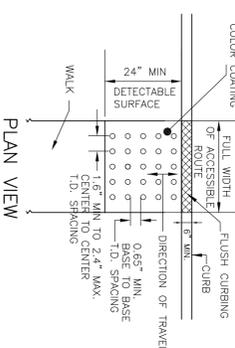
BASED ON THE FIELD SURVEY, THE MEAN ELEVATION BETWEEN THE EAKES AND THE ROOF PEAK = (846.6' PEAK + 846.6' EAK)/2 = 846.1' FT. EL.

THE EXISTING AVERAGE GRADE AS MEASURED AT 4 POINTS ALONG THE FOUNDATION IS 831.8 FT. EL.

THEREFORE, THE EXISTING BUILDING HEIGHT EQUALS: 846.1 FT. EL. - 831.8 FT. EL. = 10.3 FT. < 35 FT. MAX. COMPRES



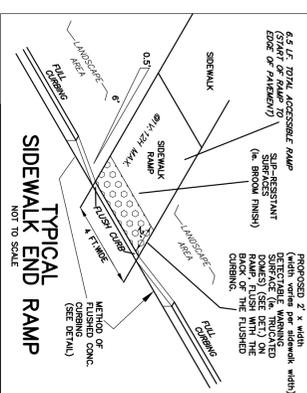
GRANITE BLOCK CURB AND FLUSHED CURB DETAIL
MORRIS COUNTY FIG. 500-9 AND 500-11



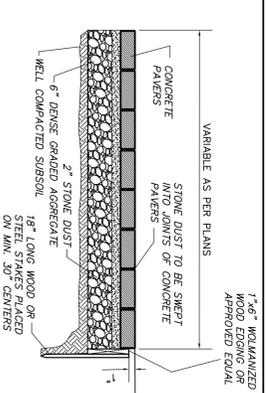
DETECTABLE WARNING SURFACE
NOT TO SCALE

NOT TO SCALE

NOTES:
1. DETECTABLE WARNING SURFACE TO CONFORM WITH N.J.A.C. 17:27-7 AND ALL APPLICABLE BUILDING CODES.
2. DETECTABLE WARNING SURFACE TO BE INSTALLED ON ACCESSIBLE WALKS.



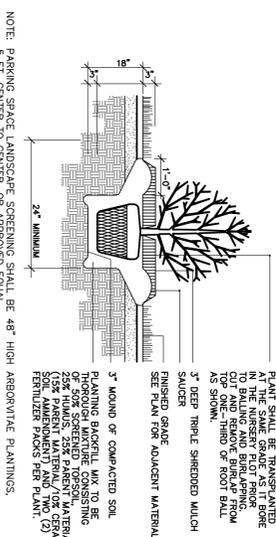
TYPICAL SIDEWALK RAMP
NOT TO SCALE



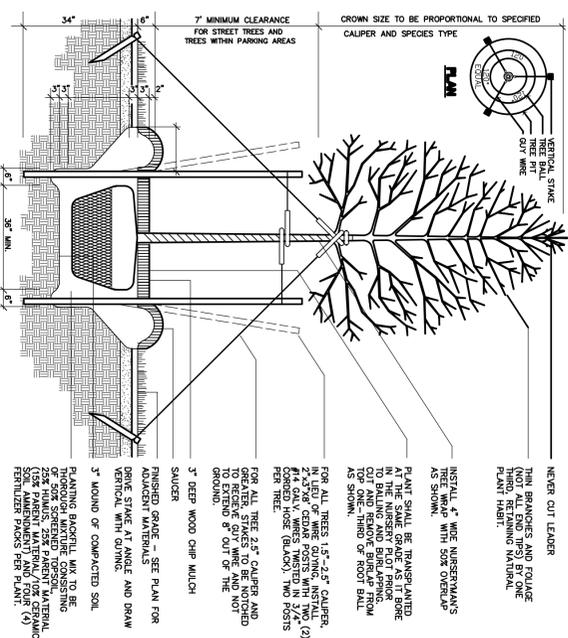
BRICK PAVER SIDEWALK - QUARRY RED 1/2\"/>

NOTE: Provided by the Applicant.

DECIDUOUS AND EVERGREEN SHRUB PLANTING DETAIL
NOT TO SCALE



NOTE: PARKING SPACE LANDSCAPE SCREENING SHALL BE 48\"/>

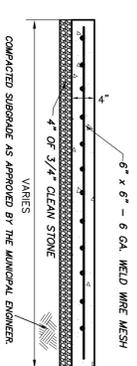


SHADE TREE PLANTING DETAIL
NOT TO SCALE

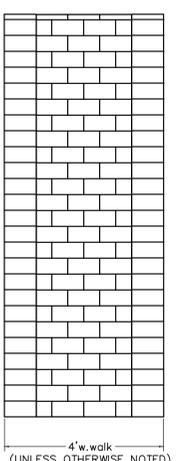
PLANTING NOTES:

1. ALL PLANTING BEDS SHALL RECEIVE 3\"/>

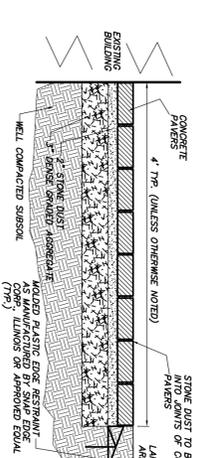
COMPUTER PAD DETAIL
NOT TO SCALE



NOTE: PAD TO BE 4\"/>



(UNLESS OTHERWISE NOTED)



CONCRETE BRICK PAVER WALK DETAIL
NOT TO SCALE

YAWACCONE & ALDRICH, LLC
Civil Engineers & Land Surveyors & Professional Planners
460 MAIN STREET, P.O. BOX 459
CHESTER, NEW JERSEY 07930
PHONE: 908-679-6646
FAX: 908-679-8591
N.J. STATE BOARD OF PROFESSIONAL ENGINEERS
& ARCHITECTS No. 422235500

CANDICE J. DAVIS
N.J. PROFESSIONAL ENGINEER
NO. 246294327000 DATE 02/09/20

PROJECT TITLE:
THE CAR WASH AT CHESTER, LLC AT SITE PLANS
MINOR IMPROVEMENTS PROJECT
45 MARLE AVENUE
LOT 5 ~ BLOCK 131
TAX MAP SHEET No. 10
BOROUGH OF CHESTER
MORRIS COUNTY, NEW JERSEY

SHEET TITLE:
CONSTRUCTION DETAIL SHEET

DRAWN BY: CJD DATE: FEB. 05, 2020
CHECKED BY: SCALE: AS SHOWN
W.O. 219096
F.B.
FILE: 17499/219096/cjd/
219096-steph.dwg

COMP FILE: 17499/219096/cjd/
219096-steph.dwg
SHEET 6 OF 6

THE CAR WASH AT CHESTER, LLC SITE PLANS

PROPOSED MINOR IMPROVEMENTS

45 MAPLE AVENUE LOT 5 - BLOCK 131

CHESTER BOROUGH TAX MAP SHEET No. 10

BOROUGH OF CHESTER, MORRIS COUNTY, NJ

PROPERTY OWNERS WITHIN 200 FT. OF LOT 5, BLOCK 131
BASED ON A CERTIFIED LIST FROM THE BOROUGH OF CHESTER TAX COLLECTOR, DATED NOVEMBER 4, 2018.

BLOCK	LOT	LOCATION	NAME & ADDRESS
128	2	24 MAPLE AVENUE	DPF CHESTER LT. MARVIN F. ROER & CO 3520 PIEDMONT RD NE #410 ATLANTA, GA 30305
128	4	141-205 ROUTE 206	DPF CHESTER LT. MARVIN F. ROER & CO 3520 PIEDMONT RD NE #410 ATLANTA, GA 30305
131	2	24 SEMINARY AVENUE	HAMILTONS CHESTER LLC CHESTER, NJ 07930
131	3	30 SEMINARY AVENUE	HAROLD HARPH PO BOX 121 OLDWICK, NJ 08858
131	4	65 MAPLE AVENUE	CHESTER 65 LLC 4 GALLOWAY HOLDEN, NJ 07733
131	6	25 MAPLE AVENUE	438700 PNC BANK CHESTER J005 PO BOX 182726 COLUMBUS, OH 43218-2726
131	7	65 ROUTE 206	HARGIS & JOHNSON INC / FTA LLC HOLDEN, NJ 07733

PUBLIC SERVICE ELECTRIC & GAS
 80 PARK PLACE
 NEWARK, NJ 07101

NU AMERICAN WATER CO.
 1000 W. 12TH ST.
 CHERRY HILL, NJ 08034

APPROVED BY THE BOROUGH
 OF CHESTER LAND USE BOARD:

Chairperson	Date
Secretary	Date
Borough Engineer	Date

- REFERENCE NOTES:**
1. LOT 5, BLOCK 131, AS SHOWN ON BOROUGH OF CHESTER TAX MAP SHEET NO. 10.
 2. ADJACENT BUILDINGS, ROADWAYS AND WOODED AREAS ARE TAKEN FROM THE NJ-GEOGRAPHIC DATA: <https://www.mapdata.com/Products/MapData>, 2015 Natural Imagery.
 3. CHESTER BOROUGH ZONE DISTRICTS AS SHOWN PER THE CHESTER BOROUGH ZONING MAP, 2015. THE SUBJECT PROPERTY IS LOCATED WITHIN THE B-3 ZONE AND HISTORIC DISTRICT PRESERVATION OVERLAY.
 4. THE SUBJECT PROPERTY IS LOCATED WITHIN THE UDEP HIGHLANDS PLANNING ZONE.

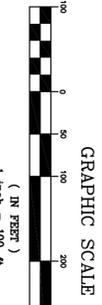
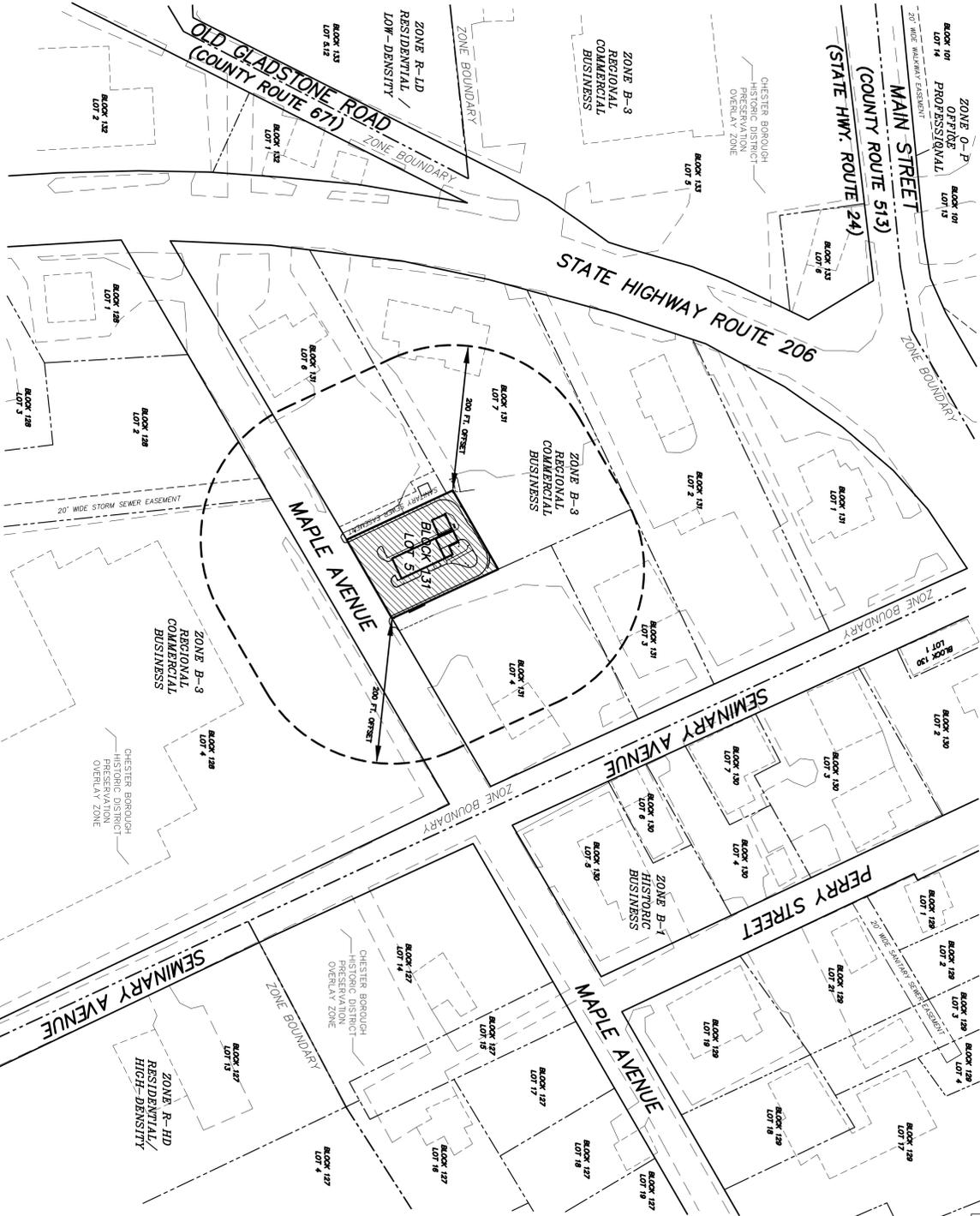
PROPERTY OWNER
 45 MAPLE LLC
 c/o DANIEL SQUIRE
 383 RIDGEVALE AVENUE
 EAST HANOVER, NJ 07936-1445
 201-625-2443

APPLICANT
 THE CAR WASH AT CHESTER LLC
 c/o DANIEL SQUIRE
 615 PAULSADE AVENUE, APT-5
 CLIFTSIDE PARK, NJ 07010
 201-625-2443

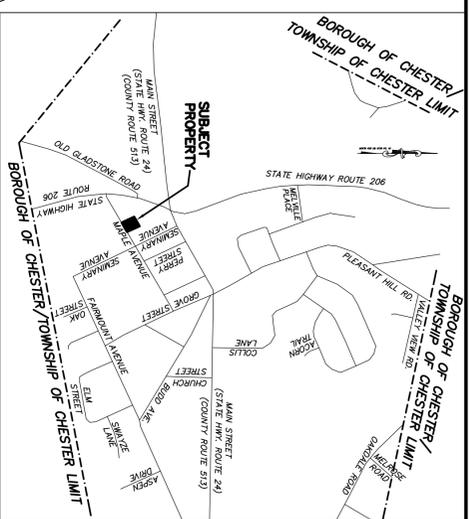
ATTORNEY
 ANTHONY J. SPASARO, ESQ.
 444 EAST MAIN STREET
 PO BOX 836
 CHESTER, NJ 07930
 908-879-8400

CIVIL ENGINEER
 CANDICE J. DAVIS, PE
 YANNAKONE, VILLA & ALDRICH, LLC
 460 MAIN STREET, PO BOX 459
 CHESTER, NEW JERSEY 07930
 908-879-6646

LAND SURVEYOR
 CHRISTOPHER J. ALDRICH, PLS
 YANNAKONE, VILLA & ALDRICH, LLC
 P.O. BOX 459
 CHESTER, NEW JERSEY 07930
 908-879-6646



AREA MAP
 GRAPHIC SCALE



KEY MAP
 SCALE: 1"=1000'

PROJECT DRAWING INDEX

SHEET NO.	DESCRIPTION
1 of 6	TITLE SHEET
2 of 6	EXISTING CONDITIONS & ENVIRONMENTAL CONSTRAINTS PLAN SHEET
3 of 6	ZONING PLAN SHEET
4 of 6	CIRCULATION & SIGNAGE PLAN SHEET
5 of 6	PLANNING PLAN SHEET
6 of 6	CONSTRUCTION DETAIL SHEET

W/D.	2190966
F.B.	
CHECKED BY:	GCY
SCALE:	1"=100'

PROJECT TITLE:
THE CAR WASH AT CHESTER, LLC SITE PLANS
 PROPOSED MINOR IMPROVEMENTS
 45 MAPLE AVENUE
 LOT 5 - BLOCK 131
 BOROUGH OF CHESTER
 MORRIS COUNTY, NEW JERSEY

TITLE SHEET

DRAWN BY:	DATE:
CJD	FEB. 05, 2020

NO.	DATE	REVISION

FILE:	2190966-si-pln-1-Hatched.dwg
COMP FILE:	I:/ENG/2190966/2190966-si-pln-1-Hatched.dwg

YANAKONE & ALDRICH, LLC
 Civil Engineers & Land Surveyors & Professional Planners
 460 MAIN STREET, P.O. BOX 459
 CHESTER, NEW JERSEY 07930
 PHONE: 908-879-6646
 FAX: 908-879-8591
 N.J. STATE BOARD OF P.E. & L.S. CERT. NO. 24624794500
 N.J. STATE BOARD OF L.S. CERT. NO. 24624794500

CANDICE J. DAVIS
 N.J. PROFESSIONAL ENGINEER
 NO. 24624527000 DATE 02/05/20